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Mathieu Foca

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# Thèse de Doctorat

**Mathieu FOCA**

*Mémoire présenté en vue de l'obtention du  
**grade de Docteur de l'École Centrale de Nantes**  
sous le label de l'Université Nantes Angers Le Mans*

**École doctorale: Sciences pour l'Ingénieur, Géosciences, Architecture (SPIGA)**

**Discipline: Mécanique des solides, des matériaux, des structures et des surfaces**  
**Unité de recherche: institut de recherches en Génie civil et Mécanique (GeM)**

**Soutenue le vendredi 16 janvier 2015**

***On a Local Maximum Entropy interpolation approach for simulation of  
coupled thermo-mechanical problems. Application to the Rotary Frictional  
Welding process.***

## JURY

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Ces trois années et demi passées au GeM comptent à ce jour parmi les plus belles années de ma vie, riches en événements, en rencontres et, bien évidemment, riches d'un point de vue scientifique.

Je tiens tout d'abord à remercier l'ensemble des membres de mon jury pour avoir eu la gentillesse de m'accorder un peu de leur temps en ces périodes de fin et de début d'année : Pierre Villon, qui m'a fait l'honneur de présider ce jury ; Ignacio Romero et Eric Feulvarch, pour leur relecture attentive et minutieuse, leur regard critique et leurs commentaires constructifs ; Jean-Philippe Ponthot pour ses questions judicieuses lors de la soutenance et enfin Elias Cueto pour son suivi tout au long de mes années de thèse. Je remercie également Patrick Romilly, invité, pour avoir suivi ce projet en y apportant la dimension industrielle permettant de toujours avoir en vue la raison de nos recherches parfois si abstraites.

J'adresse de sincères remerciements à mon directeur de thèse, Laurent Stainier, non seulement pour m'avoir accordé sa confiance sur ce sujet passionnant mais aussi pour m'avoir permis d'aller littéralement à l'autre bout du monde. Je tiens également à témoigner toute ma reconnaissance à mon co-encadrant de thèse, Guillaume Racineux, pour sa grande gentillesse et ses conseils pratiques et avisés.

-

Un grand merci à toute l'équipe de l'équipe Structure et Simulation : Nicolas M., Claude et Antony pour leurs questions toutes plus redoutées les unes que les autres en réunion d'équipe mais également les plus pertinentes ; Eric et Pierre-Emmanuel pour leur grande patience vis-à-vis de mes talents avec Linux ; Gregory, Laurent G., Nicolas C., Pascal, Patrice et Patrick avec qui c'est toujours un plaisir de discuter et ce depuis mes années en cycle ingénieur ; et tous les autres pour leur gentillesse, Caroline, Marie, Mélissa, Alexis, Gilles et Thomas.

Une petite pensée pour tous ceux avec qui j'ai eu la chance de partager mon bureau : les grands anciens Shaopu, JC et Augusto (même si à l'arrivée seule la demoiselle a soutenue avant moi !), celle qui n'a fait qu'y passer Qian et les petits derniers Daria et, bien évidemment, mon grand ami Simon !

Un gros clin d'œil à tous les collègues et amis pas encore cités : ce petit filou de Romain, perspicace et inquiétant ; Loïc, dit Lolo d'Arabie, mon partenaire de muscu ; Kévin qui m'a tout appris ; Ophélie la machine inarrêtable ; Prashant le californien ; Matthieu et André, indisociables et toujours en train de se sacrifier/d'être sacrifiés pour que tout se déroule pour le mieux ; et tous les autres: Alina, Elia, Florent, Benoît, Bérenger, Cyril, Quentin et Olivier.

Évidemment je me dois d'avoir une pensée pour le groupe nantais des 6trouilles dont les soirées jeux et Magic ont rythmé ces années de thèse : Gaël, Simon, TT et Mélanie, Thibault, Xavier, Denis, Fred, Octavio, Antonin et Élodie, Manu et Mady. Une pensée également pour mes amis toulousains: Gurb, Geb le moche, Geb le geek, JTM, Poulpy et Julien.

Enfin, je tiens à remercier chaleureusement ma famille : Papa et Maman pour leur présence le jour de ma soutenance et leur soutien inconditionnel durant toutes ces années. Je ne saurais finir sans une douce pensée pour ma chère Marie, qui m'a toujours soutenue et qui me supporte depuis toutes ces années.



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# Chapter 1

## Introduction

*This first chapter will present the context and objectives of the thesis. It begins with some explanations about the driven Rotary Frictional Welding (RFW) and the current Finite Element (FEM) models. It also contains a state of the art related to the most common meshless methods and their limitations to model the friction welding processes.*

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# 1 Direct Driven Rotary Friction Welding Process (RFW)

## 1.1 Context

This thesis is a part of an industrial project aiming to develop a direct driven Rotary Friction Welding (RFW) machine able to weld large parts. This project involved two companies, ACB [<http://www.acb-ps.com>] and Jeumont Electric [<http://www.jeumontelectric.com>], and three laboratories, the GeM (Institut de recherche en Genie civil et Mécanique) institute, the IMN (Institut des matériaux de Nantes) institute and the LAMPA (Laboratoire Arts et Métiers ParisTech d'Angers) institute.

The project was build around two observations. First, the current industrial RFW machines are not sufficient to cover the needs of the aeronautical industry such as the need to weld large parts together in order to build larger engines for instance. Then, even if there are a lot of manufacturer in the world, only two of them can be considered as main manufacturers. This situation has been considered as an opportunity to propose an alternative to existing industrial solutions.

A consortium was created in order to build this alternative. It has three main objectives. The first one is to develop a pole of industrial and academical expertise by the collaboration of the previously mention organisms. The second one is to develop RFW applications in the aeronautical industry by building larger machines with a more accurate control of the process than any other market offer. The last one is to propose an industrial alternative allowing ACB to become a global actor in the friction welding domain thanks to the experience of the consortium gained since 2007 with the work on the linear friction welding process.

In this context, a meshless method based on the Local Maximum Entropy (LME) approach is proposed to deal with coupled thermo-mechanical phenomena including contact conditions and large deformations. This type of approach avoids the issues related to the remeshing steps in the Finite Element Method (FEM) and the subsequent degradation of the temperature fields for example. This last point is crucial in the case of the RFW modeling to ensure the quality of the prediction of the metallurgical state of the material after the welding.

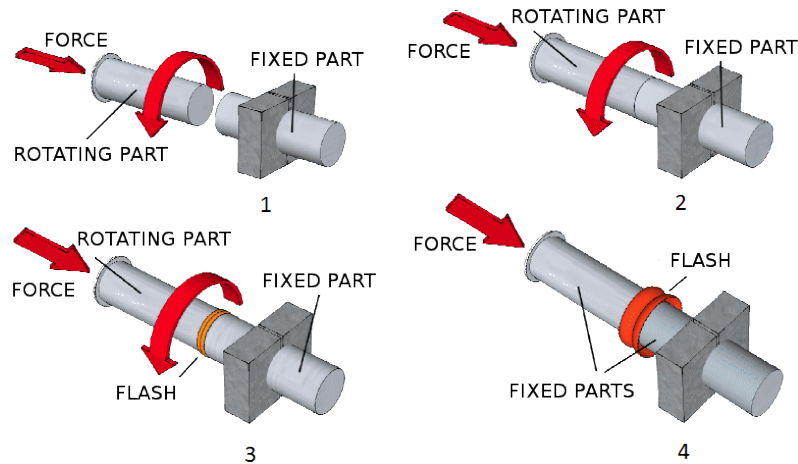
## 1.2 The RFW process

Rotary Friction Welding (RFW) has been industrially exploited for more than sixty years and is still the most widely used of friction technologies. The first patent about RFW was taken out in 1891 in the US. However, the first experiments with the welding of two metals were only realized in 1940. In 1945, Caterpillar has



developed the inertial RFW process which uses a pre-loaded inertia wheel to have the power required by the welding. In 1960, RFW was used in the US to weld the oil drilling rods and the first industrial machine was made in 1961. In the 70s, the direct driven RFW was used in aeronautical industry and in the 90s, the inertial RFW process was used for the largest parts.

The RFW process allows to weld two different parts of which one at least has a cylindrical shape. The direct driven process may be divided into four stages:



**Figure 1.1:** The four stages of the RFW process

#### Stage 1:

The two parts are put in the machine: the cylindrical one in a spindle and the other one in a stationary vice. The spindle is attached to the motor and is put to a prescribed rotary speed.

#### Stage 2:

Initial contact. A pre-determined pressure is applied at the bottom of the rotating part and puts the two parts in contact. The contact is maintained until the friction removes the surface irregularities. The frictional heating starts.

#### Stage 3:

The mechanical energy is converted into heat energy and mechanical deformation to increase the temperature at the welding interface. This temperature is generally very close to the melting temperature and a plasticized layer is created at the interface. The incompressibility of this layer leads to the formation of flashes, which leads to the component shortening, also called *upset*.

#### Stage 4: Forge.

The motor is stopped. A larger force is applied in order to end the welding

by removing the last remaining impurities via the flashes. The temperature is decreasing. After a certain time, the components are unloaded and removed from the machine for the final cooling-down.

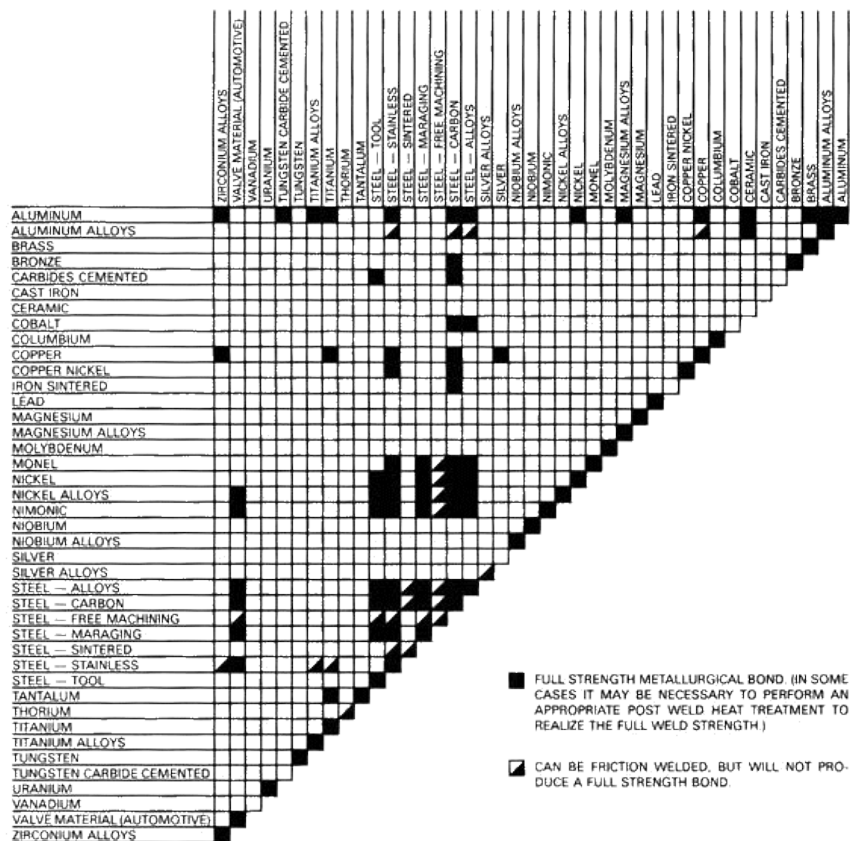
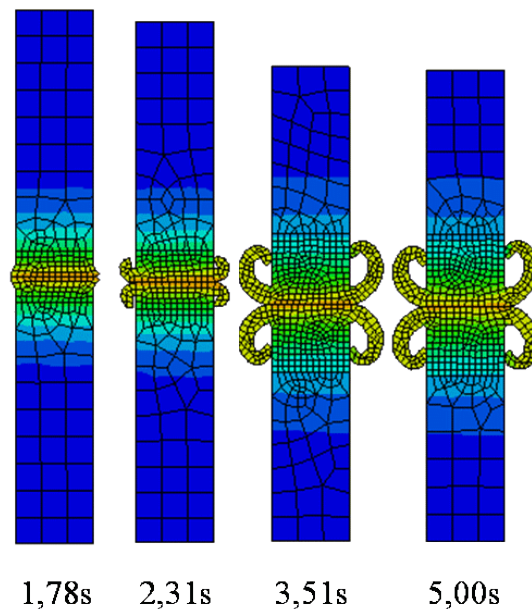


Figure 1.2: Weldable materials [www.ardindustries.com]

Compared to other welding processes, such as TIG (Tungsten Inert Gas), EBW (Electron Beam Welding), plasma and laser, RFW has many advantages. First of all, there is no porosity defect at the interface since they are all ejected in the flashes. Then, there is no material addition and the welding is of forged type. Hence the characteristics of the welding are in general at least as good as the one of the welded materials separately. The microstructure of the welding area is of wrought type, which means that *unweldable* material can be welded such as Nickel superalloy. It is also possible to weld together different materials as shown on figure 1.2.

### 1.3 Current RFW modeling using Finite Elements Method (FEM)

The mechanical computation of the RFW process using a Finite Element (FEM) approach is now relatively well known. At the beginning, studies used to consider only a purely thermal model. In 1990, Sluzalec [Słuzalec, 1990] modeled the driven RFW considering the thermo-mechanical coupling. During the last twenty years, numerous researches have also been conducted considering the coupling for the inertia RFW process [Soucail et al., 1992] [Moal and Massoni, 1995]. Commercial codes, like Abaqus for instance [Simulia Abaqus, 2010], propose the necessary scripts to "easily" run the coupled thermo-mechanical computation using FEM.



**Figure 1.3:** Remeshing steps during Abaqus computation of RFW [Simulia Abaqus, 2010]

The results obtained with FEM are close enough to experimental data to be considered as a reliable prediction, even when the welding of two dissimilar materials is modeled [D'Alvise et al., 2002].

However, standard FEM may not be the most adapted method to model this process since they also present some drawbacks. Indeed, the thermo-mechanical coupling has a very important role and requires a high accuracy level, which the FEM might not reach because of the large deformations.

To handle the large deformations, FEM computations proceed to a certain number of remeshing steps as shown on figure 1.3. Studies put in evidence that in general, adaptive meshes may give reasonable prediction but may also totally miss certain physi-

cal phenomena such as wrinkling and shear band phenomena [Lee and Bathe, 1994]. Moreover, the accuracy on the thermal field might not be sufficient for metallurgy purpose.

## 2 Objectives of this work

Control of the direct driven RFW process is achieved by manipulating three main operating parameters: the rotary speed  $\omega(t)$ , the torque  $N(t)$  and finally the pressure  $P(t)$  applied on the bottom of the rotary part. The use of experimental trials to determine optimum process parameters has proven effective but a substantial number of test pieces are required to find the good settings. The empirical approach is expensive and lacks flexibility in term of geometry and design optimization.

Finite Element computations are effective but require a substantial number of remeshing steps, which represent a loss of time but also a loss of accuracy because of the transfer of the different fields from the old mesh to the new one.

On the other hand, meshless methods have some superior features compared to conventional grid-based methods such as FEM. The most relevant argument in this context is that meshless methods can easily handle large deformation because of the absence of any mesh: they need no mesh generation, suffer no mesh distortion nor mesh alignment sensitivity and no remeshing are necessary. The other main advantages are the high smoothness of the shape functions, the generally better convergence and finally that the volumetric locking may be alleviated by tuning the dilation parameter of the kernel function.

The aim of this study is to develop and propose a meshfree approach based on the Local Maximum Entropy method (LME). This approach must be able to predict the temperature, heat affected zone dimensions, material shortening (upset), stress, residual stress and strain fields. This code should bring to the industry a representation correct enough to be able to perform microstructure studies.

LME basis functions have been used in computational mechanics for only a few years but already provide very impressive results, especially in the modeling of high velocity impacts in the Optimal Transportation Method (OTM) of Li et al. [Li et al., 2010]. Therefore, it is expected that a LME approach should give good results in the modeling of the RFW.

## 3 Meshless approximations

A multitude of different meshless methods has been published in the last decades. Despite the number of methods, there are significant similarities between many of them and the major difference lies in the manner of constructing the approximation of a single function  $u(\boldsymbol{x})$  in the domain based on a set of scattered nodes. This section will describe the most common approximations used for meshless methods.

Cueto and Chinesta [Cueto and Chinesta, 2013] recently made a review about the efficiency of different meshless methods for the simulation of material forming.

### 3.1 Smooth Particle Hydrodynamics (SPH)

Smooth Particle Hydrodynamics (SPH) has been introduced in 1977 by Lucy [Lucy, 1977] and Gingold and Monaghan [Gingold and Monaghan, 1977] and is one of the earliest particle method. SPH basically consists in approximating a function  $u(x)$  on a domain  $\Omega$  by a convolution.

$$u^h(x) = \int_{\Omega} C_p \Phi\left(\frac{\mathbf{x} - \mathbf{y}}{\rho}\right) u(\mathbf{y}) d\mathbf{y} \quad (1.1)$$

where  $u^h$  is the approximation of  $u$ ,  $\Phi$  is a function defined on a compact usually called *weight*, *window function* or *kernel function* and  $\rho$  is the dilation parameter.  $C_p$  is called normality property of the window function and is a constant such that

$$\int_{\Omega} C_p \Phi\left(\frac{\mathbf{y}}{\rho}\right) d\mathbf{y} = 1 \quad (1.2)$$

The discrete SPH form is obtained using numerical quadratures

$$u^h(\mathbf{x}) = \sum_a C_p \Phi\left(\frac{\mathbf{x} - \mathbf{x}_a}{\rho}\right) u_a \omega_a \quad (1.3)$$

where  $\mathbf{x}_a$  are the nodes called particles and  $\omega_a$  are the weights of the associated quadrature.  $u_a \equiv u(\mathbf{x}_a)$  is the value of the original function  $u$  at particle  $\mathbf{x}_a$ .

The discrete window function is defined by

$$w(\mathbf{x} - \mathbf{x}_a, \rho) = C_p \Phi\left(\frac{\mathbf{x} - \mathbf{x}_a}{\rho}\right) \quad (1.4)$$

Therefore, the SPH meshless approximation can be defined by

$$u_h(\mathbf{x}) = \sum_a N_a(\mathbf{x}) u_a \quad (1.5)$$

with the approximation basis  $N_a(\mathbf{x}) = w(\mathbf{x} - \mathbf{x}_a, \rho) \omega_a$ .

The window function  $\Phi$  plays an important role in this meshless method. It is required to satisfy the following conditions [Monaghan, 1982]:

- $w(\mathbf{x} - \mathbf{x}_a, \rho) > 0$  on a subdomain  $\Omega_a$  of  $\Omega$ ;
- $w(\mathbf{x} - \mathbf{x}_a, \rho) = 0$  outside of the subdomain  $\Omega_a$ ;
- $w(\mathbf{x} - \mathbf{x}_a, \rho) \rightarrow \delta(\mathbf{x} - \mathbf{x}_a)$  the Dirac function as  $\rho \rightarrow 0$ ;
- $w(\cdot, \rho)$  is a monotonically decreasing function.

The three most commonly used window functions are the cubic spline, Gaussian and quartic spline. For example, let  $d_a = \|\mathbf{x} - \mathbf{x}_a\|$ ,  $r = d_a/d_a^{max}$  with  $d_a^{max}$  the size of the support of the  $a$ -th particle. The window functions can be written as a function of the normalized radius  $r$ :

$$\text{cubic spline : } w(r) = \begin{cases} \frac{2}{3} - 4r^2 + 4r^3 & \text{for } r \leq \frac{1}{2} \\ \frac{4}{3} - 4r + 4r^2 - \frac{4}{3}r^3 & \text{for } \frac{1}{2} < r \leq 1 \\ 0 & \text{for } r > 1 \end{cases} \quad (1.6a)$$

$$\text{Gaussian: } w(r) = \begin{cases} \frac{\exp(-(\alpha r)^2) - \exp(-\alpha^2)}{1 - \exp(-\alpha^2)} & \text{for } r \leq 1 \\ 0 & \text{for } r > 1 \end{cases} \quad (1.6b)$$

$$\text{quartic spline: } w(r) = \begin{cases} 1 - 6r^2 + 8r^3 - 3r^4 & \text{for } r \leq 1 \\ 0 & \text{for } r > 1 \end{cases} \quad (1.6c)$$

Even if the continuous form of SPH meshless approximation is zeroth-order complete and most of the window functions satisfy higher order consistency condition, the discrete SPH form is not able of even reproducing constant fields and hence, is not a partition of unity. The conditions for zeroth-order and first order consistency are:

$$\sum_a N_a(\mathbf{x}) = 1 \quad (1.7a)$$

$$\sum_a N_a(\mathbf{x}) \mathbf{x}_a = \mathbf{x} \quad (1.7b)$$

Consistency is generally necessary for convergence, which directly contributes to the interpolation error. Corrections must be made to the window functions for convergence and accuracy of SPH method. Numerous improvements have been developed to fulfill the completeness: Johnson-Beissel correction [Johnson and Beissel, 1996], Randles-Libersky correction [Randles and Libersky, 1996], Krongauz-Belytschko correction [Belytschko et al., 1998], Monaghan's symmetrization on derivative approximation [Monaghan, 2005], etc. However, most of the analysis related to the convergence, stability and accuracy properties are based on uniformly distributed particles and the results obtained by such analysis are often limited to idealized circumstances. In general cases implying large deformations where the particles are disordered, the obtained results may not always be reliable since the effect of the particle irregularity on the accuracy of the solution is not very clear.

Moreover, the SPH shape functions are not interpolant since  $u^h(x_a) \neq u_a$  and they do not verify the Kronecker delta property:

$$N_a(\mathbf{x}_b) \neq \delta_{ab} \quad (1.8)$$

Thus it is not easy to apply boundary conditions on nodes, which is absolutely necessary for the use of meshless method in solid mechanics, without special techniques.

### 3.2 Moving Least Squares (MLS)

The objective of the Moving Least Squares (MLS) is to obtain an approximation based on an array of nodes in the considered domain, but with high accuracy and high order of completeness. The MLS method was first introduced in curve and surface fitting by Lancaster and Salkauskas [Lancaster and Salkauskas, 1981] and then in solid mechanics in the diffuse element method by Nayroles et al. [Nayroles et al., 1992]. Other studies and applications are made in the Element-Free Galerkin (EFG) methods by Belytschko et al. [Belytschko et al., 1994]. Basically MLS is about approximating  $u(\mathbf{x})$  through a polynomial of order  $m$  with non-constant coefficients in the domain:

$$u^h(\mathbf{x}) = \sum_i^m \mathbf{p}_i(\mathbf{x}) \mathbf{b}_i(\mathbf{x}) = \mathbf{p}^T(\mathbf{x}) \mathbf{b}(\mathbf{x}) \quad (1.9)$$

where  $p_0(\mathbf{x}) = 1$  and  $p_i(\mathbf{x})$  are monomials in the space coordinates  $\mathbf{x} = (x_1, \dots, x_d)^T \in \mathbb{R}^d$  so that the basis is complete. For example, in one dimension, a complete polynomial  $p$  of order  $m$  is

$$\mathbf{p}(x) = (1, x, x^2, \dots, x^m)^T \quad (1.10)$$

and  $\mathbf{b}(x)$  is given by

$$\mathbf{b}(x) = (b_0(x), b_1(x), b_2(x), \dots, b_m(x))^T \quad (1.11)$$

where the unknown parameters  $b_i(x)$  at any given point are determined by minimizing the difference of a weighted discrete  $L_2$ -norm between the local approximation at that point and the nodal parameter  $u_a$  as follow:

$$\begin{aligned} \inf_{\mathbf{b}} J \text{ with } J &= \sum_{a=1}^n w(\mathbf{x} - \mathbf{x}_a) (u^h(\mathbf{x}_a) - u_a)^2 \\ &= \sum_{a=1}^n w(\mathbf{x} - \mathbf{x}_a) (\mathbf{p}^T(\mathbf{x}_a) \mathbf{b}(x) - u_a)^2 \end{aligned} \quad (1.12)$$

where  $w(\mathbf{x} - \mathbf{x}_a)$  is a window function with compact support as mentioned in SPH methods,  $n$  is the number of nodes in the neighborhood of  $\mathbf{x}$  and  $u_a$  is the nodal value of  $u$  at  $\mathbf{x} = \mathbf{x}_a$ . The stationarity of  $J$  in equation 1.12 with respect to  $\mathbf{b}(x)$  leads to a linear relationship between  $\mathbf{b}(x)$  and  $u_a$ :

$$\mathbf{b}(\mathbf{x}) = \mathbf{A}^{-1}(\mathbf{x}) \mathbf{B}(\mathbf{x}) \mathbf{u} \quad (1.13)$$

where

$$\mathbf{A}(\mathbf{x}) = \sum_{a=1}^n w_a(\mathbf{x}) \mathbf{p}(\mathbf{x}) \otimes \mathbf{p}(\mathbf{x}) \quad (1.14a)$$

$$\mathbf{B}(\mathbf{x}) = [w_1(\mathbf{x})\mathbf{p}(\mathbf{x}_1), w_2(\mathbf{x})\mathbf{p}(\mathbf{x}_2), \dots, w_n(\mathbf{x})\mathbf{p}(\mathbf{x}_n)]^T \quad (1.14b)$$

$$\mathbf{u} = [u_1, u_2, \dots, u_n]^T \quad (1.14c)$$

Hence, the approximation  $u^h(\mathbf{x})$  is

$$u^h(\mathbf{x}) = \sum_{a=1}^n N_a(\mathbf{x}) u_a \quad (1.15)$$

where the shape function related to node  $a$  is

$$N_a(\mathbf{x}) = \sum_{i=0}^m \mathbf{p}_i(\mathbf{x}) (\mathbf{A}^{-1}(\mathbf{x}) \mathbf{B}(\mathbf{x}))_{ia} \quad (1.16)$$

The MLS approximation perfectly reproduces all the polynomials in  $\mathbf{p}(\mathbf{x})$ . Therefore, the consistency of order  $m$  is satisfied by the MLS approximation if the basis is complete in the polynomials of order  $m$ . However, the main drawback of the MLS is the efficiency: in order to obtain an accurate shape function and compute the inverse matrix  $\mathbf{A}^{-1}$ , the number of nodes in the influence domain is usually much greater than the number of monomials in the polynomial basis, especially in two or three dimensions. It also must be noted that MLS does not satisfy the Kronecker delta criterion either, therefore associated shape functions are not interpolant: the nodal parameters  $u_a$  are not the nodal values of  $u^h(\mathbf{x}_a)$  and the approximation on the boundary of the domain may depend on the nodal data of interior nodes. Just like SPH, this property makes the imposition of boundary conditions more complicated than with finite elements.

### 3.3 Reproducing Kernel Particle Method (RKPM)

The Reproducing Kernel Particle Method (RKPM) introduced by Liu et al. [Liu et al., 1995] is an improvement of the continuous SPH approximation. A correction function  $C(\mathbf{x}, \mathbf{y})$  is introduced into equation 1.1 in order to increase the order of completeness of the approximation

$$u^h(\mathbf{x}) = \int_{\Omega} C(\mathbf{x}, \mathbf{y}) \Phi\left(\frac{\mathbf{x} - \mathbf{y}}{\rho}\right) u(\mathbf{y}) d\mathbf{y} \quad (1.17)$$

where  $K(\mathbf{x}, \mathbf{y}) = C(\mathbf{x}, \mathbf{y}) w(\mathbf{x} - \mathbf{y}, \rho)$  is defined such that the approximation is  $m$ -th order consistent. Suppose that  $\mathbf{p}(\mathbf{x})$  is a complete array of monomial up to  $m$ -th order. Then any  $m$ -th order polynomials can be written

$$u(\mathbf{x}) = \mathbf{p}^T(\mathbf{x}) \mathbf{b} \quad (1.18)$$



where  $\mathbf{b}$  are the unknown coefficients. Then

$$\int_{\Omega} \mathbf{p}(\mathbf{y}) w(\mathbf{x} - \mathbf{y}, \rho) u(\mathbf{y}) d\mathbf{y} = \left( \int_{\Omega} \mathbf{p}(\mathbf{y}) \mathbf{p}^T(\mathbf{y}) w(\mathbf{x} - \mathbf{y}, \rho) d\mathbf{y} \right) \mathbf{b} \quad (1.19)$$

which is a system of equations for  $\mathbf{b}$ . A substitution in equation 1.18 leads to

$$u^h(\mathbf{x}) = \mathbf{p}^T(\mathbf{x}) \left( \int_{\Omega} \mathbf{p}(\mathbf{y}) \mathbf{p}^T(\mathbf{y}) w(\mathbf{x} - \mathbf{y}, \rho) d\mathbf{y} \right)^{-1} \int_{\Omega} \mathbf{p}(\mathbf{y}) w(\mathbf{x} - \mathbf{y}, \rho) u(\mathbf{y}) d\mathbf{y} \quad (1.20)$$

Thus the correction function is

$$C(\mathbf{x}, \mathbf{y}) = \mathbf{p}^T(\mathbf{x}) \left( \int_{\Omega} \mathbf{p}(\mathbf{y}) \mathbf{p}^T(\mathbf{y}) w(\mathbf{x} - \mathbf{y}, \rho) d\mathbf{y} \right)^{-1} = \mathbf{p}^T(\mathbf{x}) (\mathbf{M}(\mathbf{x}))^{-1} \mathbf{p}(\mathbf{y}) \quad (1.21)$$

The discrete RKPM form is obtained using numerical quadratures

$$\begin{aligned} u^h(\mathbf{x}) &= \sum_{a=1}^n C(\mathbf{x}, \mathbf{x}_a) w(\mathbf{x} - \mathbf{x}_a) u_a \omega_a \\ &= \mathbf{p}^T(\mathbf{x}) (\mathbf{M}(\mathbf{x}))^{-1} \sum_{a=1}^n \mathbf{p}(\mathbf{x}_a) w(\mathbf{x} - \mathbf{x}_a) u_a \omega_a \end{aligned} \quad (1.22)$$

Numerical integration is also required to evaluate the moment matrix  $\mathbf{M}(\mathbf{x})$

$$\begin{aligned} \mathbf{M}(\mathbf{x}) &= \int_{\Omega} w(\mathbf{x} - \mathbf{y}, \rho) \mathbf{p}(\mathbf{y}) \mathbf{p}^T(\mathbf{y}) d\mathbf{y} \\ &= \sum_{a=1}^n w(\mathbf{x} - \mathbf{x}_a, \rho) \mathbf{p}(\mathbf{x}_a) \mathbf{p}^T(\mathbf{x}_a) \omega_a \end{aligned} \quad (1.23)$$

Just like the SPH approximation, RKPM does not verify the Kronecker delta property and special techniques are needed to impose essential boundary conditions such as described by Chen [Chen et al., 1997].

### 3.4 Natural Element Method (NEM)

The Natural Element Method (NEM) introduced by Sukumar [Sukumar, 1998] [Sukumar and Moran, 1999] [Sukumar et al., 2001] is the first really successful attempt for a method free of error in the interpolation of the essential variable along the boundary. The NEM was originally a Galerkin method in which the interpolation was achieved through natural neighbor (NN) methods [Sibson, 1980] [Sibson, 1981] [Belikov et al., 1997] [Belikov and Semenov, 2000]. More recently, a book has been written by Chinesta et al. [Chinesta et al., 2013] about the use of the NEM in the simulation of structures and processes.

As Element Free Galerkin Method (EFGM) [Belytschko et al., 1994], NEM constructs the connectivity of each material point using the concept of Delaunay triangulation. The advantage of the NEM is the good accuracy provided despite the distortion of this triangulation, as proved by Sukumar [Sukumar, 1998]. The Delaunay triangulation [Delaunay, 1934] of a cloud of nodes  $X = \{\mathbf{x}_1, \mathbf{x}_2, \dots, \mathbf{x}_n\} \subset \mathbb{R}^d$  is the unique triangulation of the cloud that satisfy the so-called circumcircle criterion. The dual structure of the Delaunay triangulation is the Voronoi diagram. It is composed by a tessellation of the space into cells of the form

$$T_{x_a} = \{\mathbf{x} \in \mathbb{R}^n \mid \forall b \neq a, d(\mathbf{x}, \mathbf{x}_a) < d(\mathbf{x}, \mathbf{x}_b)\} \quad (1.24)$$

where  $T_a$  represents the Voronoi cell and  $d(\cdot, \cdot)$  the Euclidean distance. The most popular natural neighbor (NN) interpolant is due to Sibson [Sibson, 1981]. Consider the second-order Voronoi cell:

$$T_{x_a, x_b} = \{\mathbf{x} \in \mathbb{R}^n \mid \forall c \notin \{a, b\}, d(\mathbf{x}, \mathbf{x}_a) < d(\mathbf{x}, \mathbf{x}_b) < d(\mathbf{x}, \mathbf{x}_c)\} \quad (1.25)$$

Then the Sibsonian shape function is defined as

$$\phi_a(\mathbf{x}) = \frac{A_a(\mathbf{x})}{A(\mathbf{x})} \quad (1.26)$$

where  $A_a(\mathbf{x})$  is the Lebesgue measure of the cell  $T_{x_a, x_a}$  and  $A(\mathbf{x}) = \sum_{a=1}^n A_a(\mathbf{x})$  is the Lebesgue measure of the cell  $T_x$ .

The NN interpolation defined by these shape functions has remarkable properties. First, the partition of unity property is verified:

$$\sum_{a=1}^n \phi_a(\mathbf{x}) = 1 \quad (1.27)$$

The partition of unity property implies the non-negativity and convexity of trial function:

$$0 \leq \phi_a(\mathbf{x}) \leq 1 \quad (1.28)$$

Moreover, the cardinal, or Delta-Kronecker, property is fulfilled and the shape functions are interpolant:

$$\phi_a(\mathbf{x}_b) = \delta_{ab}, \quad u^h(\mathbf{x}_a) = u_a \quad (1.29)$$

The linear completeness property (it can exactly reproduces linear displacement fields) is also verified:

$$\mathbf{x} = \sum_{a=1}^n \phi_a(\mathbf{x}) \mathbf{x}_a \quad (1.30)$$

The supports of the shape functions are compact, and hence a local interpolation scheme is realized. Finally, the shape functions are smooth ( $\mathcal{C}^1$ ) everywhere except at the nodes where they are simply continuous ( $\mathcal{C}^0$ ).

Despite the interesting properties of NN interpolation, the main drawback of NEM is its high computational cost, especially for Sibson interpolation. An analysis of the computational cost of several meshless method was made by Alfaro et al. [Alfaro et al., 2006]. It is shown that mesh distortion could lead to important inaccuracies when using FEM while Sibson interpolation is several orders of magnitude heavier to compute than traditional piecewise polynomial shape functions for finite elements. However, in non-linear computations, while frequent Newton-Raphson iterations are needed, the relative cost of shape function is obscured by the cost of updating tangent stiffness matrix.

### 3.5 Material Points Method (MPM)

The Material Point Method (MPM) introduced by Sulsky et al. [Sulsky et al., 1993] [Sulsky et al., 1995] is an extension of the particle-in-cell method [Evans et al., 1957]. This method is not about interpolating but about being able to consider the history dependency of the constitutive equations in a broad class of engineering problems such as penetration, impact or large rotations of solid bodies. It is a discrete solution procedure for computational solid mechanics which is generalized using a variational form and a Petrov-Galerkin discretization scheme, resulting in a family of methods named the Generalized Interpolation Material Point (GIMP) [Bardenhagen and Kober, 2004]. The essential idea is to take advantage of both the Eulerian and Lagrangian methods, which can be summarized as follows.

If a continuous material under a purely mechanical loading is considered, the governing differential equations can be derived from the conservation equation of mass

$$\frac{d\rho}{dt} + \rho \nabla \cdot \mathbf{v} = 0 \quad (1.31)$$

and the conservation equation of momentum

$$\rho \mathbf{a} = \nabla \cdot \mathbf{s} + \rho \mathbf{b} \quad (1.32)$$

supplemented with a suitable constitutive equation and kinematic relations between strain and displacement. In the equations 1.31 and 1.32,  $\rho(\mathbf{x}, t)$  refers to the mass density,  $\mathbf{v}(\mathbf{x}, t)$  is the velocity,  $\mathbf{a}(\mathbf{x}, t)$  is the acceleration,  $\mathbf{s}(\mathbf{x}, t)$  is the Cauchy stress tensor,  $\mathbf{b}(\mathbf{x}, t)$  is the specific body force and  $\mathbf{x}$  is the current position at time  $t$  of any material point. The key difference among different spatial discretization methods is the way in which the gradient and divergence terms are calculated.

The MPM discretizes a continuous body by using a finite set of  $n_p$  material points in the original configuration that are tracked throughout the deformation process. Let  $\mathbf{x}_i^t$ ,  $i \in \{1, \dots, n_p\}$  denote the current position of material point  $p$  at time  $t$ . Each material point at time  $t$  has an associated mass  $M_p$ , density  $\rho_p^t$ , velocity  $\mathbf{v}_p^t$ , Cauchy

stress tensor  $\mathbf{s}_p^t$ , strain  $\mathbf{e}_p^t$  and any other internal state variables necessary for the constitutive model. Thus these material points provide a Lagrangian description of the continuous body. Since each material point contains a fixed amount of mass for all time, equation 1.31 is automatically satisfied. At each time step, the information from the material points is mapped to a background computational mesh (grid). This mesh covers the computational domain of interest and is chosen for computational convenience. After the information is mapped from the material points to the mesh nodes, the discrete formulation of equation 1.32 can be obtained on the nodes.

By using the same procedure used in FEM, the weak form of equation 1.32 is given by

$$\int_{\Omega} \rho \mathbf{w} \cdot \mathbf{a} \, d\Omega = - \int_{\Omega} \rho \mathbf{s}^s : \nabla \mathbf{w} \, d\Omega + \int_{S^c} \rho \mathbf{c}^s \cdot \mathbf{w} \, dS + \int_{\Omega} \rho \mathbf{w} \cdot \mathbf{b} \, d\Omega \quad (1.33)$$

where  $\mathbf{w}$  is the test function,  $\mathbf{s}^s = \frac{1}{\rho} \mathbf{s}$  is the specific stress,  $\Omega$  is the current configuration of the material,  $S^c$  is the part of the boundary subject to a traction and  $\mathbf{w}$  is assumed to be zero on the boundary with a prescribed displacement.

Since the whole continuum body is described with the use of a finite set of material points, the mass density at a general position  $\mathbf{x}$  is

$$\rho(\mathbf{x}, t) = \sum_{p=1}^{n_p} M_p \delta(\mathbf{x} - \mathbf{x}_p^t) \quad (1.34)$$

where  $\delta$  is the Dirac delta function (with dimension of the inverse of a volume). In other words, the mass is non-null only at the material points. The substitution into equation 1.33 converts the integrals into the sums of quantities evaluated at the material points:

$$\begin{aligned} & \sum_{p=1}^{n_p} M_p [\mathbf{w}(\mathbf{x}_p^t, t) \cdot \mathbf{a}(\mathbf{x}_p^t, t)] \\ &= \sum_{p=1}^{n_p} M_p \left[ -\mathbf{s}^s(\mathbf{x}_p^t, t) : \nabla \mathbf{w}|_{\mathbf{x}_p^t} + \mathbf{w}(\mathbf{x}_p^t, t) \cdot \mathbf{c}^s(\mathbf{x}_p^t, t) h^{-1} + \mathbf{w}(\mathbf{x}_p^t, t) \cdot \mathbf{b}(\mathbf{x}_p^t, t) \right] \end{aligned} \quad (1.35)$$

with  $h$  the thickness of the boundary layer. The interactions among different material points are reflected only through the gradient terms, and a suitable set of material points must be chosen to represent the boundary layer. In the MPM, a background computational mesh is required to calculate the gradient terms. To do so, we suppose that a computational mesh is constructed, using 2-node cells in 1D,

3-node cells in 2D and 4-node cells in 3D for example. These cells are then employed to define standard FEM nodal basis functions  $N_i(\mathbf{x})$  related to the spatial nodes  $\mathbf{x}_i(t)$  with  $i \in \{1, \dots, n\}$  where  $n$  is the total number of mesh nodes. The coordinates of any material point in a cell can be represented by

$$\mathbf{x}_p^t = \sum_{i=1}^n \mathbf{x}_i^t N_i(\mathbf{x}_p^t) \quad (1.36)$$

In the same way, the displacement of any material point in a cell are defined by the nodal displacement  $\mathbf{u}_i^t(t)$ . Therefore:

$$\mathbf{u}_p^t = \sum_{i=1}^n \mathbf{u}_i^t N_i(\mathbf{x}_p^t) \quad (1.37)$$

Since the same basis functions are used for both spatial coordinates and displacements, kinematic compatibility demands that the basis functions advect with the material, as in the updated Lagrangian framework. In other words, the *material* time rates of the basis functions must be zero. Hence, it follows that the velocity and acceleration of any material point in a cell are represented by

$$\mathbf{v}_p^t = \sum_{i=1}^n \mathbf{v}_i^t N_i(\mathbf{x}_p^t) \quad (1.38)$$

and

$$\mathbf{a}_p^t = \sum_{i=1}^n \mathbf{a}_i^t N_i(\mathbf{x}_p^t) \quad (1.39)$$

where  $\mathbf{v}_p^t$  and  $\mathbf{a}_p^t$  are respectively the nodal velocities and accelerations. The test function is also defined by

$$\mathbf{w}_p^t = \sum_{i=1}^n \mathbf{w}_i^t N_i(\mathbf{x}_p^t) \quad (1.40)$$

Those equations ensures that the associated vectors are continuous across the cell boundary. However, their gradients are not continuous due to the the use of linear shape functions. Substituting equations 1.39 and 1.40 into equation 1.35 yields

$$\begin{aligned} & \sum_{p=1}^{n_p} \mathbf{w}_i^t \cdot \sum_{p=1}^{n_p} m_{ij}^t \mathbf{a}_j^t \\ &= - \sum_{p=1}^{n_p} \mathbf{w}_i^t \cdot \sum_{p=1}^{n_p} M_p \mathbf{s}^{s,t} \cdot \nabla N_i|_{\mathbf{x}_p^t} + \sum_{i=1}^n \mathbf{w}_i^t \cdot \mathbf{c}_i^t + \sum_{i=1}^n \mathbf{w}_i^t \cdot \mathbf{b}_i^t \end{aligned} \quad (1.41)$$

at time  $t$ . In equation 1.41, the consistent mass matrix is given by

$$m_{ij}^t = \sum_{p=1}^{n_p} M_p N_i(\mathbf{x}_p^t) N_j(\mathbf{x}_p^t) \quad (1.42)$$

with corresponding lumped nodal masses

$$m_i^t = \sum_{p=1}^{n_p} M_p N_i(\mathbf{x}_p^t) \quad (1.43)$$

The discrete specific traction is then

$$\mathbf{c}_i^t = \sum_{p=1}^{n_p} M_p \mathbf{c}_p^{s,t} h^{-1} N_i(\mathbf{x}_p^t) \quad (1.44)$$

with  $\mathbf{c}_p^{s,t} = \mathbf{c}_p^s(\mathbf{x}_p^t, t)$  while the specific body force is discretized as

$$\mathbf{b}_i^t = \sum_{p=1}^{n_p} M_p \mathbf{b}_p^t N_i(\mathbf{x}_p^t) \quad (1.45)$$

with  $\mathbf{b}_p^{s,t} = \mathbf{b}_p^s(\mathbf{x}_p^t, t)$ . One should notice that if  $\mathbf{b}$  is a known function of position and time, as gravity for instance, then the nodal body force can be computed directly by  $\mathbf{b}_i^t = \mathbf{b}(\mathbf{x}_p^t, t) m_i^t$ .

Since the  $\mathbf{w}_i^t$  are arbitrarily chosen except where the components of displacement are prescribed, for a lumped mass matrix, equation 1.41 simply becomes:

$$m_i^t \mathbf{a}_i^t = (\mathbf{f}_i^t)^{int} + (\mathbf{f}_i^t)^{ext} \quad (1.46)$$

where the internal force is given by:

$$(\mathbf{f}_i^t)^{int} = - \sum_{p=1}^{n_p} M_p \mathbf{s}_p^{s,t} \cdot \mathbf{G}_i(\mathbf{x}_p^t) \quad (1.47)$$

with  $\mathbf{s}_p^{s,t} = \mathbf{s}^s(\mathbf{x}_p^t, t)$ ,  $\mathbf{G}_i(\mathbf{x}_p^t) = \nabla N_i|_{\mathbf{x}_p^t}$  and the external force is

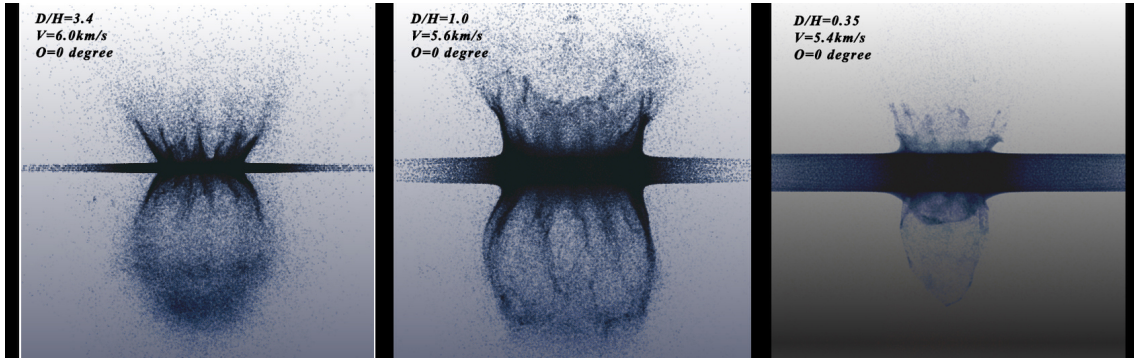
$$(\mathbf{f}_i^t)^{ext} = \mathbf{c}_i^t + \mathbf{b}_i^t \quad (1.48)$$

As shown in the previous equations, the information is mapped from material points to the nodes of the cell containing these points through the use of shape functions.

The key feature of MPM is the use of the same set of nodal basis functions for both the mapping from material points to cell nodes to solve equation 1.46 and the mapping from cell nodes to material points to update the material point information for the next time step. This method has the advantage to be able to handle large deformation but just like the methods presented previously, it is very difficult to apply boundary conditions [Chen and Brannon, 2002]. Finally, MPM can only be used for isothermal mechanics analysis.

### 3.6 Optimal Transportation Meshfree (OTM) method: an example of LME interpolation use

The Optimal Transportation Meshfree (OTM) method has been developed by Li et al. [Li et al., 2010] a few years ago to simulate general solid and fluid flows, including fluid-structure interaction, and is mostly used to perform high velocity impact simulations such as the ones presented on figure 1.4. It combines the MPM approach and the maximum entropy interpolation (see chapter 2) in order to generalize the Benamou-Brenier differential formulation of optimal mass transportation problems to problems including arbitrary geometries and constitutive behaviors.



**Figure 1.4:** Three high velocity impact simulations using OTM on a metallic target with distinct thicknesses.

The algorithm used to run a mechanical simulation is the following:

**Algorithm** OTM - Elastic solid flow [Li et al., 2010]

1. Initialization: Set  $k = 0$ , initialize nodal coordinates  $x_{a,-1}$ ,  $x_{a,0}$ , shape functions  $N_{a,0}$ , material points coordinates  $x_{p,0}$ , volumes  $v_{p,0}$ , densities  $\rho_{p,0}$  and deformation gradients  $F_{p,0}$ .
2. Compute mass matrix  $M_k$ , linear moment  $l_k$  and internal forces  $f_k$  (same as MPM).
3. Update nodal coordinates:

$$x_{k+1} = x_k + (t_{k+1} - t_k) M_k^{-1} \left( l_k + \frac{t_{k+1} - t_{k-1}}{2} f_k \right)$$

4. Update material point coordinates using the LME basis functions  $\phi$ :

$$x_{p,k+1} = \phi_{h,k \rightarrow k+1}(x_{p,k})$$

5. Update material point volumes:

$$v_{p,k+1} = \det \nabla \phi_{h,k \rightarrow k+1}(x_{p,k}) v_{p,k}$$

6. Update material point mass densities:

$$\rho_{p,k+1} = \frac{m_p}{v_{p,k+1}}$$

7. Update material point deformation gradients and right Cauchy-Green deformation tensors:

$$F_{p,k+1} = \nabla \phi_{h,k \rightarrow k+1}(x_{p,k}) F_{p,k}$$

$$C_{p,k+1} = F_{p,k+1}^T F_{p,k+1}$$

8. Recompute shape functions  $N_{a,k+1}(x_{p,k+1})$  and derivatives  $\nabla N_{a,k+1}(x_{p,k+1})$  from updates nodal set.

9. Reset  $k \leftarrow k + 1$ . If  $k = N$  exit. Otherwise go to (2).

Our work and the OTM method have a lot in common: the use of material points and the LME basis functions (see chapter 2). The main difference is that the OTM method has a dynamic approach based on the optimal transportation principle whereas our work is based on a Galerkin approach (the mass matrix is assumed to be constant). The current limitations of the OTM method are that it is somewhat restricted to use an explicit time integration scheme and cannot simulate conduction problems and thus cannot perform coupled thermo-mechanical analyses.

## 4 Thesis outline

In this thesis, we aim at building a meshless method using the Local Maximum Entropy (LME) interpolation combined to a strongly coupled thermo-mechanical variational formulation. The objective is to be able to model coupled thermo-mechanical problems with large deformation which may include complex boundary conditions such as contact and/or convection.

In chapter 2, we present an energy-based variational modeling of strongly coupled thermo-mechanical problems adapted to a meshless method. We first present the time-continuous evolution problem and its variational formulation, followed by the time-discrete (or incremental) variational formulation and finally the time-space-discrete variational formulation considering the meshless environment. Then some flow stress models are presented considering the variational framework.

In chapter 3, the LME interpolation approach is described. The construction of LME shape functions and how the LME-based meshless method is working are



detailed. A particular attention is paid to the localization of the material points. Then, some features are exposed and finally, we present some adjustments needed to work in an updated-Lagrangian framework.

In chapter 4, test cases are proposed in order to verify our implementation of the LME-based meshless method. From a simple conduction test to a tensile test considering conduction, convection, large deformation and coupled thermo-viscoplasticity passing by some classical benchmark tests as the Taylor bar or the patch test, the possibilities, but also the limits, of our implementation are exposed.

Finally, the chapter 5 is dedicated to the modeling of the RFW process. We are presenting some specific modules implemented such as the contact in the friction area and the resulting heat flux. Numerical results are then presented and analyzed, in the light of available experimental observations.

## Chapter 2

# Thermo-mechanical problem

*A variational approach is proposed for the modeling of strongly coupled thermo-mechanical problems. In this chapter, variational formulations and associated principles are described and used to establish the variational thermo-mechanical framework that is used in our implementation. Then, some flow stress models are presented and the associated stored and dissipative plastic energy are detailed.*

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# 1 Energy-based variational method and variational principles

Variational principles have played an important role in mechanics for several decades [Lanczos, 1970] and have been mostly developed for conservative systems. The most eminent examples are Hamilton's principle [Hamilton, 1834] in dynamics and the principle of minimum potential energy in statics. Some variational principles are also used for dissipative systems, such as principles of maximum plastic dissipation for limit analysis.

From a mathematical but also numerical point of view, variational approaches present many attractive features such as unicity, convergence and stability of the formulations. This has motivated a lot of interest following the pioneering work of Biot [Biot, 1956]. It also well concerns isothermal settings such as elasto-viscoplasticity [Comi et al., 1991] [Ortiz and Stainier, 1999] or isothermal brittle and ductile damage [Francfort and Marigo, 1998] [Balzani and Ortiz, 2012] and coupled thermo-elastic and thermo-visco-elastic problems [Herrmann, 1960] [Batra, 1989].

Basically, a variational principle is an optimization approach used to derive the balance and the evolution equations of a boundary value problem. The most popular principles are Hamilton's and Veubeke-Hu-Washizu's.

Let us define  $\phi(t)$  as the transformation mapping describing the state of the system at the time  $t$  [Marsden and Hughes, 1994]. We then seek to write variational principles determining the evolution of the system, in a dynamical or quasi-constant setting.

## 1.1 Hamilton's principle

Hamilton's principle [Hamilton, 1834] is a statement that the dynamics of the physical system can be determined by a unique function, the Lagrangian, which contains all physical information concerning the system and the forces acting on it. Hamilton's principle states that the true evolution  $\phi(t)$  of a system between two specified times  $t_1$  and  $t_2$  is a stationary point for the functional

$$\mathcal{S}(\phi) = \int_{t_1}^{t_2} L(\phi, \dot{\phi}, t) dt \quad (2.1)$$

where  $L(\phi, \dot{\phi}, t)$  is the Lagrangian. In other words, the evolution  $\phi(t)$  of a system is the solution of

$$\forall \delta\phi, D_\phi[S(\phi)](\delta\phi) = 0 \quad (2.2)$$

## 1.2 Hu-Washizu-Fraeijs de Veubeke's principle

Hu-Washizu-Fraeijs de Veubeke's principle [Washizu, 1955] [de Veubeke, 1972] [Hu, 1984] is the canonical principle of static elasticity. It depends on three independent fields which are: the configuration  $\phi$ , the deformation gradient  $\mathbf{F}$  and the Piola stress tensor  $\mathbf{P}$ . The associated functional  $\mathcal{H}(\phi, \mathbf{F}, \mathbf{P})$  is defined by

$$\mathcal{H}(\phi, \mathbf{F}, \mathbf{P}) = \int_{B_0} [W(\mathbf{F}^T \mathbf{F}) + P \cdot (\nabla \phi - \mathbf{F})] dV - \int_{B_0} \rho_0 \bar{\mathbf{b}} \cdot \phi dV - \int_{\partial_\sigma B_0} \bar{\mathbf{t}} \cdot \phi dA \quad (2.3)$$

where  $W$  is the inner elastic strain energy depending on the Cauchy-Green tensor  $\mathbf{C} = \mathbf{F}^T \mathbf{F}$  to ensure the independence of the coordinate system,  $\bar{\mathbf{b}}$  is the body forces (gravity for instance) and  $\bar{\mathbf{t}}$  the imposed force applied on the part  $\partial_\sigma B_0 = \partial B_0 \setminus \partial_\phi B_0$  of the external surface of the body. Then, the quasi-static evolution of the system is described by a sequence of equilibrium status, each satisfying the variational principle

$$\inf_{\phi, \mathbf{F} \text{ adm.}} \sup_{\mathbf{P}} \mathcal{H}(\phi, \mathbf{F}, \mathbf{P}) \quad (2.4)$$

An admissible configuration  $\phi$  must verify the essential boundary conditions  $\phi = \bar{\phi}$  on  $\partial_\phi B_0$ . An admissible deformation gradient  $\mathbf{F}$  must verify the material impenetrability condition  $\det \mathbf{F} > 0$ . Every tensor can be an admissible stress tensor  $\mathbf{P}$ .

The Gâteaux derivatives  $D_\phi [\mathcal{H}(\phi, \mathbf{F}, \mathbf{P})](\delta\phi)$ ,  $D_{\mathbf{F}} [\mathcal{H}(\phi, \mathbf{F}, \mathbf{P})](\delta\mathbf{F})$  and  $D_{\mathbf{P}} [\mathcal{H}(\phi, \mathbf{F}, \mathbf{P})](\delta\mathbf{P})$  respectively yield the static equilibrium equation 2.5, the constitutive equation 2.6 and finally the compatibility equation 2.7.

$$\nabla \cdot \mathbf{P}^T + \rho_0 \bar{\mathbf{b}} = 0 \quad (2.5)$$

$$\mathbf{P} = 2\mathbf{F} \frac{\partial W(\mathbf{C})}{\partial \mathbf{C}} \quad (2.6)$$

$$\mathbf{F} = \nabla_0 \cdot \phi \quad (2.7)$$

Thus, the stress Piola-Kirchhoff tensor is given by

$$\mathbf{S} = \mathbf{F}^{-1} \mathbf{P} = 2 \frac{\partial W(\mathbf{C})}{\partial \mathbf{C}} \quad (2.8)$$

## 1.3 Hellinger-Reissner's principle

Hellinger-Reissner's principle is obtained from the previous principle. Let us introduce the complementary strain energy density, obtained by a Legendre transformation of the energy density

$$W^*(\mathbf{S}) = \sup_{\mathbf{C}} \left[ \frac{1}{2} \mathbf{S} : \mathbf{C} - W(\mathbf{C}) \right] \quad (2.9)$$

The inverse transformation is

$$W(C) = \sup_{\mathbf{S}} \left[ \frac{1}{2} \mathbf{S} : C - W^*(\mathbf{S}) \right] \quad (2.10)$$

If the compatibility equation  $\mathbf{F} = \nabla \phi$  is verified, then we obtain the functional

$$\mathcal{H}(\phi, \mathbf{S}) = \int_{B_0} \left[ \frac{1}{2} \mathbf{S} : (\nabla \phi^T \nabla \phi) - W^*(\mathbf{S}) \right] dV - \int_{B_0} \rho_0 \bar{b} \phi dV - \int_{\partial_\sigma B_0} \bar{t} \phi dA \quad (2.11)$$

and the boundary value problem has the following variational form:

$$\inf_{\phi \text{ adm.}} \sup_{\mathbf{S} \text{ adm.}} \mathcal{H}(\phi, \mathbf{S}) \quad (2.12)$$

#### 1.4 Minimum potential energy principle

As an example, let us consider how the minimum potential energy principle is used to build the variational modeling.

If the compatibility equation 2.5 and the constitutive equation 2.6 are assumed verified, then the functional 2.3 becomes [Marsden and Hughes, 1994]

$$\begin{aligned} \mathcal{H}(\phi) &= \int_{B_0} W(\nabla_0 \phi^T \nabla_0 \phi) dV - \int_{B_0} \rho_0 \bar{b} \phi dV - \int_{\partial_\sigma B_0} \bar{t} \phi dA \\ &= \mathcal{U}(\phi) - \mathcal{W}(\phi) \end{aligned} \quad (2.13)$$

with the potential strain energy

$$\mathcal{U}(\phi) = \int_{B_0} W(\nabla_0 \phi^T - \nabla_0 \phi) dV \quad (2.14)$$

and the energy from external forces

$$\mathcal{W}(\phi) = \int_{B_0} \rho_0 \bar{b} \phi dV + \int_{\partial_\sigma B_0} \bar{t} \phi dA \quad (2.15)$$

where  $\rho_0$  is the mass density,  $\bar{b}$  is the body force,  $\nabla_0$  is the material gradient and  $\bar{t}$  the imposed force. Assuming the compatibility condition and constitutive relations are satisfied, the boundary value problem via variational principle is described by

$$\inf_{\phi \text{ adm.}} (\mathcal{U}(\phi) - \mathcal{W}(\phi)) \quad (2.16)$$

Its stationary point with respect to  $\phi$  corresponds to the conservation of momentum in elasticity (see equation 2.19).

As illustrated by many examples, the variational form of equations is very convenient for the numerical simulations, and the uniqueness and existence of the solutions in the problem are easily analyzed from a mathematical point of view. By means of this energy-based variational method, we emphasize that a physical problem is transformed to a mathematical optimization, and then a series of optimization algorithms can be applied in the analysis of physical fields.

## 1.5 Balance equations in local coupled thermo-dynamical model

The variational formulation of a coupled thermo-mechanical problem includes the three classical conservation equations of mechanics:

- Conservation of mass

$$\rho \det \mathbf{F} = \rho_0 \quad (2.17)$$

where  $\rho$  is the mass density in the deformed configuration,  $\rho_0$  is the mass density in the initial configuration and  $\mathbf{F}$  is the deformation gradient.

- Conservation of linear momentum

$$\rho_0 \ddot{\phi} = \nabla_0 \cdot \mathbf{P}^T + \rho \mathbf{b} \quad (2.18)$$

where  $\mathbf{P}$  is the first Piola-Kirchhoff (or Piola) stress tensor and  $\mathbf{b}$  represents the applied bulk forces per mass unit.

- Conservation of angular momentum

$$\mathbf{P}\mathbf{F}^T = \mathbf{F}\mathbf{P}^T \quad (2.19)$$

The thermo-mechanical coupling involves two more conservation laws which represent the laws of thermo-dynamics:

- Conservation of energy (first law of thermo-dynamics)

$$\rho_0 T \dot{\eta} = \mathbf{P} : \dot{\mathbf{F}} + \mathbf{Y}^d : \dot{\mathbf{Z}} - \nabla_0 \cdot \mathbf{H} + \rho_0 Q \quad (2.20)$$

where  $\mathbf{H}$  is the nominal (Lagrangian) heat flux vector,  $Q$  the applied bulk heat source (per mass unit) and  $\eta$  is the internal entropy density (per mass unit).

- Clausius-Duhem Inequality (second law of thermo-dynamics)

$$T\dot{\Gamma} = \mathcal{D}_{\text{int}} - \frac{1}{T}\mathbf{H} \cdot \nabla T \geq 0 \quad (2.21)$$

where  $T$  is the absolute temperature and  $\dot{\Gamma}$  denotes the net entropy production rate.

Furthermore, a boundary value problem is described by the above governing equations including material constitutive relations and compatibility conditions. According to the variational method, by multiplying a small constrained but arbitrary variation  $l \in \mathcal{V} = \{\mathbf{l} \in \mathbb{R}^3 | \mathbf{l} = \mathbf{0} \text{ on the boundaries}\}$ , the weak form of the problem can be obtained.

For instance, consider the following mechanical problem:

$$\begin{aligned} \nabla_0 \cdot \mathbf{P}^T + \rho_0 \mathbf{B} &= 0 \\ \mathbf{P} \cdot \mathbf{n} &= \boldsymbol{\tau} \text{ on } L \end{aligned} \quad (2.22)$$

where  $\boldsymbol{\tau}$  is the traction on the boundary  $L$ . The weak form is defined by

$$\text{Find } \boldsymbol{\phi} \in \mathcal{P} \text{ such that } \forall \mathbf{l} \in \mathcal{V}, G(\boldsymbol{\phi}, \mathbf{l}) = 0 \quad (2.23)$$

where

$$G(\boldsymbol{\phi}, l) = \int_V (\mathbf{P} : \nabla \mathbf{l} - \rho_0 \mathbf{B} \cdot \mathbf{l}) dV - \int_L \boldsymbol{\tau} \cdot \mathbf{l} dL \quad (2.24)$$

with  $\boldsymbol{\phi} \in \mathcal{P} = \{\boldsymbol{\phi} \in \mathbb{R}^3 | \boldsymbol{\phi} = \bar{\boldsymbol{\phi}} \text{ in the boundaries}\}$ . If the physical fields and  $\mathbf{l}$  are  $\mathcal{C}^1$ , the weak form is equivalent to the strong form. In view of the weak form, Marsden and Hughes [Marsden and Hughes, 1994] pointed out that there is a potential  $E$  such that  $\mathbf{D}_\phi E(\boldsymbol{\phi}) \cdot \mathbf{l} = G(\boldsymbol{\phi}, l)$  if, and only if,

$$\forall \boldsymbol{\phi} \in \mathcal{P}, \forall (\mathbf{l}, \boldsymbol{\xi}) \in \mathcal{V}^2, \mathbf{D}_1 G(\boldsymbol{\phi}, l) \cdot \boldsymbol{\xi} = \mathbf{D}_1 G(\boldsymbol{\phi}, \boldsymbol{\xi}) \cdot \mathbf{l} \quad (2.25)$$

and its corresponding form is

$$E(\boldsymbol{\phi}) = \int_0^1 G(t\boldsymbol{\phi}, \boldsymbol{\phi}) dt \quad (2.26)$$

Thereby, a formulation embodying equilibrium equations, material behaviors and boundary condition is built in the variational framework. Even if the weak form of conservation laws seems to be a basis to build the variational pseudo-potential, the symmetry still has to be guaranteed.

In 1999, Ortiz and Stainier [Ortiz and Stainier, 1999] obtained a variational formulation for general viscoplastic solids with respect to different dissipative relations in finite deformation regime. They developed a constitutive update modeling as an optimization to a scalar function with a set of internal variables, including Hemholtz free energy, conjugate inelastic potential and viscous part. The associated constitutive updates can be defined by minimization of an incremental pseudo-potential about deformation over the time step. This work represents a new and active research area, the applications of this variational structure to general dissipative materials are continuously developed [Stainier et al., 2002]. For instance, constitutive visco-elastic formulations are as following provided to embody the non-linear viscous behavior based on this theoretical framework [Stainier et al., 2005] [Fancello et al., 2006] [Mosler and Bruhns, 2009] [Weinberg et al., 2006].



By considering temperature effects, a variational formulation of thermo-mechanical boundary-value problems was proposed by Yang et al. [Yang et al., 2006]. They introduced the equilibrium temperature in the formula, thus making the weak form symmetric. In addition, the characteristic of this formulation is that it allows to describe the thermal and mechanical balance equations, including irreversible and dissipative behaviors, as an optimization of an energy-like variational form. In addition, beyond unifying a wide range of constitutive models in a common framework, the variational formulation also presents the interesting mathematical properties, like symmetry of its bilinear form, which is an important feature compared to the alternative coupled thermo-mechanical formulations. By applying this variational formulation, Stainier [Stainier and Ortiz, 2010] successfully presented an experimental validation of three thermo-viscoplastic materials: aluminum alloy,  $\alpha$ -Titanium and Tantalum. These theoretical conclusions will be used to build a meshless solver for finite thermo-mechanical problems such as RFW modeling.

## 2 General framework

### 2.1 Variational formulation in thermo-mechanical coupling

Thermo-mechanical couplings are a common phenomenon in solid mechanics, and associated effects are of importance in the manufacturing of parts and structures. For general dissipative materials, the thermo-mechanical coupling can easily provoke some localization zones associated with large deformation and high temperature, e.g. the formation of adiabatic shear bands [Su et al., 2014]. Their occurrence is a precursor to material macroscopic fracture. Here we are considering a strongly coupled boundary value problem. Thus the five conservation laws detailed previously should be respected. The corresponding finite constitutive equations for thermo-mechanical coupling can be given in local form as following [Yang et al., 2006]

$$\nabla_{\mathbf{0}} \cdot \mathbf{P}^T + \rho_0 \mathbf{B} = \rho_0 \dot{\mathbf{V}} \quad (2.27)$$

$$\mathbf{F} \mathbf{P}^T = \mathbf{P} \mathbf{F}^T \quad (2.28)$$

$$\dot{E} = \mathbf{P} : \dot{\mathbf{F}} + \rho_0 Q - \nabla_{\mathbf{0}} \cdot \mathbf{H} \quad (2.29)$$

$$\dot{\gamma} \equiv \rho_0 \dot{\eta} - \frac{\rho_0 Q}{T} + \nabla \cdot \frac{\mathbf{H}}{T} \geq 0 \quad (2.30)$$

where  $\mathbf{P}$  is the first Piola stress tensor,  $\mathbf{F}$  is the deformation gradient,  $\dot{\mathbf{V}}$  is the acceleration,  $\rho_0$  is the density (per mass unit) of the undeformed volume,  $\mathbf{B}$  is the body force per mass unit,  $Q$  and  $\mathbf{H}$  are the specified heat source (per mass unit) and the nominal heat flux.  $T$  is the absolute temperature. We assume the existence of  $\mathbf{H}$  and of the free energy  $W(\mathbf{F}, T)$  such that  $\eta$  is the specific entropy defined by

$$\rho_0 \eta = - \frac{\partial W}{\partial T} \quad (2.31)$$

By using Legendre-Fenchel transform, the internal energy density  $E$  is defined:

$$E = \sup_T (\rho_0 \eta T + W) \quad (2.32)$$

and has the property

$$T = \frac{\partial E}{\partial(\rho_0 \eta)} \quad (2.33)$$

Consider the general pseudo-potential dissipation  $\Delta$  defined by

$$\Delta = \Psi^*(\dot{\mathbf{Z}}, \mathbf{Z}, T) + \phi^*(\dot{\mathbf{F}}, \mathbf{F}, T) - \chi(\mathbf{H}, T) \quad (2.34)$$

where  $\Psi^*$ ,  $\phi^*$  and  $\chi$  are the kinetic potential, viscous potential (Kelvin-Voigt visco-elasticity) and conduction potential.  $\mathbf{Z}$  represents the internal variables, such as cumulated plastic strain for thermo-visco-elastic material for instance.  $\mathbf{F}$  is the deformation gradient. Let us define  $\mathbf{P}$  as the first Piola-Kirchoff stress conjugate to  $\mathbf{F}$  as

$$\mathbf{P} = \frac{\partial W}{\partial \mathbf{F}} \quad (2.35)$$

The alternate representations of constitutive relations can similarly be obtained by using the conjugate pairs of stress-strain tensors (Cauchy stress and Cauchy-Green strain, second Piola-Kirchoff stress and Green-Lagrange finite strain, etc.). Let us define  $\mathbf{Y}$  the conjugate force to cumulated plastic deformation:

$$\mathbf{Y} = -\frac{\partial W}{\partial \mathbf{Z}} \quad (2.36)$$

The evolution equations gives

$$\mathbf{Y} = \frac{\partial \Psi^*}{\partial \dot{\mathbf{Z}}} \quad (2.37)$$

where  $\Psi^*$  is a dual pseudo-potential obtained from a Legendre-Fenchel transform of kinetic potential  $\Psi$  defined by

$$\Psi^* = \sup_{\mathbf{Y}} \left\{ \mathbf{Y} \cdot \dot{\mathbf{Z}} - \Psi \right\} \quad (2.38)$$

and

$$\dot{\mathbf{Z}} = \frac{\partial \Psi}{\partial \mathbf{Y}} \quad (2.39)$$

If

$$\Psi^* \text{ is convex, } \Psi^*(\mathbf{0}) = 0, \Psi^* \geq 0 \quad (2.40)$$

the second law of thermo-dynamics is verified. Therefore for thermo-visco-plasticity material, the first law of thermo-dynamics can also be written

$$\rho_0 C \dot{T} = T \frac{\partial^2 W}{\partial T \partial \mathbf{F}} : \dot{\mathbf{F}} + T \frac{\partial^2 W}{\partial T \partial \mathbf{Z}} \cdot \dot{\mathbf{Z}} + \mathcal{D}^{int} + \rho_0 Q - \nabla_0 \cdot \mathbf{H} \quad (2.41)$$

where  $\mathcal{D}^{int} = \mathbf{Y} \cdot \dot{\mathbf{Z}}$  is the intrinsic dissipation and  $C$  is the heat capacity defined as

$$\rho_0 C = -T \frac{\partial^2 W}{\partial T^2} \quad (2.42)$$

which depends on the temperature and state variables in the physical system. As shown in [Stainier and Ortiz, 2010] in finite plastic strains, the variational formulation naturally included provides an accurate formulation of the Taylor-Quinney parameter to calculate the ratio of intrinsic dissipation converted to total plastic power.

The second law of thermo-dynamics can be simplified as

$$T\dot{\Gamma} = \mathcal{D}^{int} - \frac{1}{T^2} \mathbf{H} \cdot \nabla T \geq 0 \quad (2.43)$$

where  $\dot{\Gamma}$  is the net entropy production rate. As the kinetic potential  $\Psi^*$  is convex, if  $\chi$  is convex then the Clausius-Duhem inequality is automatically verified.

Based on the previously described thermo-dynamic framework, the energy-based variational formulation of the coupled thermo-mechanical boundary-value problem proposed by Yang [Yang et al., 2006] can be summarized. The potential for general standard dissipative materials is stated as following:

$$\begin{aligned} \Phi \left( \dot{\phi}, T, \dot{\eta}, \dot{\mathbf{Z}} \right) = & \int_B \left[ \dot{E} - \rho_0 T \dot{\eta} + \Delta \left( \frac{T}{\Theta} \dot{\mathbf{F}}, \frac{T}{\Theta} \dot{\mathbf{Z}}, -\frac{1}{T} \nabla T \right) \right] dV \\ & - \int_B \rho_0 \mathbf{B} \cdot \dot{\phi} dV - \int_{\partial_T B} \bar{\mathbf{T}} \cdot \dot{\phi} dS \\ & + \int_B \rho_0 Q \log \frac{T}{T_0} dV - \int_{\partial_\eta B} \bar{H} \log \frac{T}{T_0} dS \end{aligned} \quad (2.44)$$

where  $\bar{\mathbf{T}}$  is the applied traction over the traction boundary  $\partial_T B$ .  $\bar{H}$  is the outward heat flux over the Neumann boundary condition  $\partial_\eta B$ . In equation 2.44, the authors introduced two temperatures  $\Theta$  and  $T$ , which are respectively an *equilibrium* or *internal* temperature and an *external* temperature, necessary to recover the balance equations.  $\Theta$  is a scaling variable and can be obtained as following

$$\Theta = \frac{\partial E}{\partial(\rho_0 \eta)} \quad (2.45)$$

This variational formulation works for general dissipative materials including finite elastic and plastic deformation, rate-sensitivity, arbitrary flow and hardening rules, as well as heat conduction. In addition, the thermal and mechanical balance equations, the constitutive relations, as well as the equilibrium between the external temperature and the internal temperature can be obtained as Euler-Lagrange equations of the following variational formulation:

$$\inf_{\dot{\phi}, \dot{\mathbf{Z}}, \dot{\eta}} \sup_T \phi \left( \dot{\phi}, T, \dot{\eta}, \dot{\mathbf{Z}} \right) \quad (2.46)$$

The equilibrium derivation is well described in [Yang et al., 2006].

Energy-based variational method is an optimization strategy using a single function to describe all the intrinsic characters for the coupled thermo-mechanical boundary value problem. The stress-strain relation, as well as temperature-entropy relation do not have to be defined separately, which can directly follow from the optimization with regard to internal variables and temperature. For example, a nonlinear equation about  $\mathbf{Z}$  can be obtained to calculate equivalent plastic strain from the variational method:

$$D_{\dot{\mathbf{Z}}} \left[ \Phi \left( \dot{\phi}, T, \dot{\eta}, \dot{\mathbf{Z}} \right) \right] \left( \delta \dot{\mathbf{Z}} \right) = 0 \quad (2.47)$$

It is also a mathematical transformation of well-known return-mapping method, and more convenient in the application of mathematical algorithm. The thermo-mechanical coupling for general dissipative materials can thus be described as an optimization problem, and many mathematical algorithms, such as trust region method, Levenberg-Marquardt algorithm, are suitable to seek a minimum or maximum value with respect to physical fields. In contrast to conventional coupled thermo-mechanical problem formulation, this variational approach intrinsically yields a symmetric stiffness matrix. Indubitably, these characteristics allow the application of a broad range of mathematical algorithms, contributing to numerical efficiency in matrix storage and nonlinear programming. Furthermore, this variational formulation seamlessly works for general standard materials.

## 2.2 Discretized variational formulation

As described in Chapter 1 about the Material Point Method (MPM), the considered system is described by using a certain number of material points. These points will be tracked throughout the deformation process. At a given time  $t$ , each material point has an associated mass, density, velocity and any other internal state variable necessary for the constitutive model.

The discretization is made in two steps: first the potential is discretized in time and then discretized in space.

### 2.2.1 Time-discretization of the variational problem

The time-discretization, as detailed in [Yang et al., 2006], is used to reduce time-dependent problems to a sequence of incremental problems each characterized by a minimum principle. For instance, it has been employed to formulate incremental minimum principles for plasticity that establish a connection between non-attainment and the formation of micro-structures [Ortiz and Repetto, 1999] [Ortiz et al., 2000] [Carstensen et al., 2002] [Aubry and Ortiz, 2003]. In addition,

the time-discretization is a key step in the numerical implementation of constitutive equations.

Formally, the time-discretized incremental variational problem can be derived by recourse to minimizing path, as in deformation theories of plasticity. Let us consider a sequence of time  $t_0, \dots, t_n, t_{n+1}, \dots$  and seek to characterize the state  $(\phi, T, \dot{\eta}, \mathbf{Z})$  of the solid at each of these times. Assuming the state  $(\phi_n, T_n, \dot{\eta}_n, \mathbf{Z}_n)$  at time  $t_n$  is known, the objective is to consistently approximate the state  $(\phi_{n+1}, T_{n+1}, \dot{\eta}_{n+1}, \mathbf{Z}_{n+1})$  at time  $t_{n+1}$ . A consistent approximation means that the limits of the divided differences  $\left[ \frac{\phi_{n+1} - \phi_n}{\Delta t}, \frac{T_{n+1} - T_n}{\Delta t}, \frac{Z_{n+1} - Z_n}{\Delta t} \right]$  as  $\Delta = t_{n+1} - t_n$  tends to zero satisfy the rate field equations. The incremental functional is introduced:

$$\Phi_n(\phi_{n+1}, T_{n+1}, \dot{\eta}_{n+1}, \mathbf{Z}_{n+1}) = \inf_{\text{paths}} \int_{t_n}^{t_{n+1}} \Phi(\dot{\phi}, T, \dot{\eta}, \dot{\mathbf{Z}}) dt \quad (2.48)$$

where the subscript  $n$  means that  $\Phi_n(\phi_{n+1}, T_{n+1}, \dot{\eta}_{n+1}, \mathbf{Z}_{n+1})$  depends parametrically on the *initial* state  $(\phi_n, T_n, \dot{\eta}_n, \mathbf{Z}_n)$  at time  $t_n$ . The minimum is taken over all admissible *paths* joining times  $t_n$  and  $t_{n+1}$ . For economy notation, let:

$$G(\dot{\phi}, T) \equiv - \int_B \rho_0 \mathbf{B} \cdot \dot{\phi} dV - \int_{\partial_T B} \bar{\mathbf{T}} \cdot \dot{\phi} dS + \int_B \rho_0 Q \log \frac{T}{T_0} dV - \int_{\partial_\eta B} \bar{H} \log \frac{T}{T_0} dS \quad (2.49)$$

Then, equation 2.48 can be written as:

$$\Phi_n(\phi_{n+1}, T_{n+1}, \dot{\eta}_{n+1}, \mathbf{Z}_{n+1}) = \inf_{\text{paths}} \int_{t_n}^{t_{n+1}} \left\{ \int_B (\dot{E} - \rho_0 T \dot{\eta} + \Delta) dV + G(\dot{\phi}, T) \right\} dt \quad (2.50)$$

with  $\Delta$  evaluated as in equation 2.44.

Despite the conceptual appeal of this approach, their explicit determination can only be effected in simple cases [Ortiz and Martin, 1989]. In calculations, it suffices to identify any convenient incremental potential  $\Phi_n$  *consistent* with the fields equations. An example of a family of consistent incremental potentials is:

$$\begin{aligned} \Phi_n(\phi_{n+1}, T_{n+1}, \dot{\eta}_{n+1}, \mathbf{Z}_{n+1}) &= \int_B [(E_{n+1} - E_n) - \rho_0 T_{n+1} (\eta_{n+1} - \eta_n) + \Delta t \Delta_{n+1}] dV \\ &- \int_B \rho_0 \mathbf{B}_{n+1} \cdot (\phi_{n+1} - \phi_n) dV - \int_{\partial_T B} \bar{\mathbf{T}}_{n+1} \cdot (\phi_{n+1} - \phi_n) dS \\ &+ \int_B \Delta t \rho_0 Q_{n+1} \log \frac{T_{n+1}}{T_n} dV - \int_{\partial_\eta B} \Delta t \bar{H}_{n+1} \log \frac{T_{n+1}}{T_n} dS \end{aligned} \quad (2.51)$$

where

$$\Delta_{n+1} = \Delta \left( \frac{T_{n+1}}{T_n} \dot{\mathbf{F}}_{n+1}, \frac{T_{n+1}}{T_n} \dot{\mathbf{Z}}_{n+1}, \mathbf{G}_{n+1}, \mathbf{F}_{n+1}, \eta_{n+1}, \mathbf{Z}_{n+1} \right) \quad (2.52)$$

and

$$\dot{\mathbf{F}}_{n+1} = \frac{\mathbf{F}_{n+1} - \mathbf{F}_n}{\Delta t} \quad (2.53a)$$

$$\dot{\mathbf{Z}}_{n+1} = \frac{\mathbf{Z}_{n+1} - \mathbf{Z}_n}{\Delta t} \quad (2.53b)$$

$$\mathbf{G}_{n+1} = -\frac{\nabla T_{n+1}}{T_{n+1}} \quad (2.53c)$$

The incremental functional 2.51 may be regarded as a backward-Euler approximation of equation 2.50. Other approximations, such as trapezoidal or midpoint rules of integration, may be formulated likewise.

The consistency of the scheme has been verified by Yang et al. [Yang et al., 2006].

It appears that the dependence of the rate functional  $\Phi$  on  $\dot{\mathbf{Z}}$  and  $\dot{\eta}$ , or of the incremental functional  $\Phi_n$  on  $\dot{\mathbf{Z}}_{n+1}$  and  $\dot{\eta}_{n+1}$ , does not involve their gradients. Therefore, the minimization with respect to  $\dot{\eta}$  and  $\dot{\mathbf{Z}}$  in the rate problem, and with respect to  $\eta_{n+1}$  and  $\mathbf{Z}_{n+1}$  in the incremental problem, may be effected pointwise. This is in contrast to the dependence of the rate functional  $\Phi$  on  $\dot{\phi}$  and  $T$ , or the incremental functional  $\Phi_n$  on  $\phi_{n+1}$  and  $T_{n+1}$ , which involves their gradients. Thus, it follows that the reduced incremental functional

$$\Phi_n(\phi_{n+1}, T_{n+1}) = \inf_{\eta_{n+1}, \mathbf{Z}_{n+1}} \Phi_n(\phi_{n+1}, T_{n+1}, \eta_{n+1}, \mathbf{Z}_{n+1}) \quad (2.54)$$

is of the form

$$\begin{aligned} \Phi_n(\phi_{n+1}, T_{n+1}) &= \int_B \psi_n(\mathbf{F}_{n+1}, T_{n+1}, \mathbf{G}_{n+1}) dV \\ &\quad - \int_B \rho_0 \mathbf{B}_{n+1} \cdot (\phi_{n+1} - \phi_n) dV - \int_{\partial_T B} \bar{\mathbf{T}}_{n+1} \cdot (\phi_{n+1} - \phi_n) dS \\ &\quad + \int_B \Delta t \rho_0 Q_{n+1} \log \frac{T_{n+1}}{T_n} dV - \int_{\partial_\eta B} \Delta t \bar{H}_{n+1} \log \frac{T_{n+1}}{T_n} dS \end{aligned} \quad (2.55)$$

where

$$\begin{aligned} \psi_n(\mathbf{F}_{n+1}, T_{n+1}, \mathbf{G}_{n+1}) &= \inf_{\eta_{n+1}, \mathbf{Z}_{n+1}} [(E_{n+1} - E_n) - \rho_0 T_{n+1} (\eta_{n+1} - \eta_n) + \Delta t \Delta_{n+1}] \\ &= \mathcal{W}_n - \Delta t \chi(\mathbf{G}_{n+1}) \end{aligned} \quad (2.56)$$

because

$$\begin{aligned} \mathcal{W}_n &= \inf_{\eta_{n+1}} [(E_{n+1} - E_n) - \rho_0 T_{n+1} (\eta_{n+1} - \eta_n) + \Delta t \Delta_{n+1}] \\ &= W_{n+1} - W_n + \rho_0 \eta_n (T_{n+1} - T_n) + \Delta t \Delta_{n+1} \end{aligned} \quad (2.57)$$

where  $\chi(\mathbf{G})$  is a convex Fourier potential (as known as Biot dissipation function) and such that

$$\mathbf{H} = \frac{\partial \chi}{\partial \mathbf{G}}(\mathbf{G}) ; \quad \mathbf{G} \equiv -\frac{\nabla T}{T} \quad (2.58)$$

$\psi_n$  may be regarded as a non-equilibrium thermo-elastic energy density. The reduced minimum problem is then

$$\inf_{\phi_{n+1} T_{n+1}} \sup \Phi_n(\phi_{n+1}, T_{n+1}) \quad (2.59)$$

### 2.2.2 Space-discretization of the variational problem

Let consider a time step  $[t_n, t_{n+1}]$ . Let  $\mathcal{V}$  the space of the admissible thermo-mechanical configurations at  $t_{n+1}$ . From variational principle 2.55, it is possible obtain a space-discretization via a standard Rayleigh-Ritz approach. As for the MPM,  $n_{nodes}$  nodes and  $n_p$  material points are used to describe the domain  $B$ . Therefore,

$$\phi^h(\mathbf{x}) = \sum_{a=1}^{n_{nodes}} N_a(\mathbf{x}) \mathbf{x}_a \quad (2.60)$$

$$T^h(\mathbf{x}) = \sum_{a=1}^{n_{nodes}} N_a(\mathbf{x}) T_a \quad (2.61)$$

where  $\mathbf{x}_a$  and  $T_a$  are the positions and the temperatures of the  $n_{nodes}$  in the current configuration. The shape functions  $N_a$ , defined all over the domain  $B$ , are the Local Maximum Entropy (LME) basis functions and will be described in the next chapter. Note that the shape functions for the approximation of the displacements and the temperature are the same.

The discretized deformation gradient and temperature gradient are

$$\mathbf{F}^h(\mathbf{x}) = \sum_{a=1}^{n_{nodes}} \nabla N_a(\mathbf{x}) \mathbf{x}_a \quad (2.62a)$$

$$\mathbf{G}^h(\mathbf{x}) = -\frac{\sum_{a=1}^{n_{nodes}} \nabla N_a(\mathbf{x}) T_a}{\sum_{a=1}^{n_{nodes}} N_a(\mathbf{x}) T_a} \quad (2.62b)$$

and the reduced variational principle 2.59 becomes

$$\inf_{\phi_{n+1}^h, T_{n+1}^h} \sup_{\phi_n, T_n, \mathbf{Z}_n} \Phi_n(\phi_{n+1}^h, T_{n+1}^h; \phi_n, T_n, \mathbf{Z}_n) \quad (2.63)$$

From now on, the subscript  $n + 1$  will be omitted for readability reasons. The Dirichlet boundary conditions are applied by setting the nodal variables  $\mathbf{x}_a$  and  $T_a$  on the boundaries.

By substitution into the incremental potential 2.55, the stationarity equations are:

$\forall \delta \mathbf{x}_a$  adm.,

$$\sum_{a=1}^{n_{nodes}} \mathbf{f}_a \delta \mathbf{x}_a = \sum_{a=1}^{n_{nodes}} \left\{ \int_B [\mathbf{P}^h \nabla N_a - \rho_0 \mathbf{b} N_a] dV - \int_{\partial_T B} \bar{T} N_a dS \right\} \delta \mathbf{x}_a = 0 \quad (2.64a)$$

$\forall \delta T_a$  adm.,

$$\sum_{a=1}^{n_{nodes}} Q_a \delta T_a = \sum_{a=1}^{n_{nodes}} \left\{ \int_B [-\rho_0 \Delta \eta_{eff}^h N_a + \Delta t \mathbf{H}^h \left( \frac{\nabla N_a}{T^h} + \frac{\mathbf{G}^h}{T^h} N_a \right) + \Delta t \rho_0 Q \frac{N_a}{T_n}] dV - \int_{\partial \eta} \Delta t \bar{H} \frac{N_a}{T_n} dS \right\} \delta T_a = 0 \quad (2.64b)$$

where  $\mathbf{P}^h$ ,  $\Delta \eta_{eff}^h$  and  $\mathbf{H}^h$  are respectively defined by:

$$\mathbf{P}^h = \frac{\partial \mathcal{W}_n}{\partial \mathbf{F}}(\mathbf{F}^h, T^h; \mathbf{F}_n, T_n, \mathbf{Z}_n) \quad (2.65)$$

$$\rho_0 \Delta \eta_{eff}^h = -\frac{\partial \mathcal{W}_n}{\partial T}(\mathbf{F}^h, T^h; \mathbf{F}_n, T_n, \mathbf{Z}_n) \quad (2.66)$$

$$\mathbf{H}^h = \frac{\partial \chi}{\partial \mathbf{G}}(\mathbf{G}^h; \mathbf{F}_n, T_n, \mathbf{Z}_n) \quad (2.67)$$

The stationarity equations 2.64 lead to the discretized equation of mechanical and thermal equilibrium which can also be written:

$$\begin{cases} \mathbf{f}_a^{\text{int}} - \mathbf{f}_a^{\text{ext}} = 0 \\ Q_a^{\text{int}} - Q_a^{\text{ext}} = 0 \end{cases} \quad (a = 1, \dots, n_{nodes}) \quad (2.68)$$



where the inner and external forces and fluxes are given by:

$$\mathbf{f}_a^{\text{int}} = \int_B \mathbf{P}^h \nabla N_a \, dV \quad (2.69a)$$

$$\mathbf{f}_a^{\text{ext}} = \int_B \rho_0 \mathbf{b} N_a \, dV + \int_{\partial_T B} \bar{T} N_a \, dS \quad (2.69b)$$

$$Q_a^{\text{int}} = \int_B \left[ \rho_0 \Delta \eta_{eff}^h N_a - \Delta t \mathbf{H}^h \left( \frac{\nabla N_a}{T^h} + \frac{\mathbf{G}^h}{T^h} N_a \right) \right] dV \quad (2.69c)$$

$$Q_a^{\text{ext}} = \int_B \Delta t \frac{\rho_0 Q}{T_n} N_a \, dV - \int_{\partial_{eta} B} \Delta t \frac{\bar{H}}{T_n} N_a \, dS \quad (2.69d)$$

It is interesting to note that every integral is done over the whole domain  $B$  because the shape functions are defined all over the domain. On the contrary, if classical FEM shape functions are used, and since they are defined over elements were used, all the  $\int$ -symbol in equation 2.69 would be replaced:  $\int_B$  would become  $\cup_{e=1}^{n_{elems}} \int_{\Omega^e}$ ,  $\int_{\partial_T B}$  would become  $\cup_{e=1}^{n_{elems}} \int_{\partial\Omega^e}$  and  $\int_{\partial_{eta} B}$  would become  $\cup_{e=1}^{n_{elems}} \int_{\partial\Omega^e}$ . In reality, the LME interpolation functions will not be define over the whole domain. Since they drop relatively quickly to zero, it is possible to cut the domain and so not to have full matrices (see chapter 3).

### 3 Variational framework and flow stress models

The evolution of the yield surface is often expressed as an equation consisting of some invariant of stress and a model for the yield stress (or plastic flow stress). An example is von Mises or  $J_2$  plasticity. In those situations the plastic strain rate is calculated in the same manner as in rate-independent plasticity. In other situations, the yield stress model provides a direct means of computing the plastic strain rate. Numerous empirical and semi-empirical flow stress models are used in the computational plasticity. In the following, we will restrict ourselves to isotropic hardening and we define  $p$  as the cumulated plastic strain.

The Johnson-Cook model (JC) [Johnson and Cook, 1983] is purely empirical and is one of the most widely used. However, this model exhibits an unrealistically small strain-rate dependence at high temperatures. The Steinberg-Cochran-Guinan-Lund (SCGL) model [Steinberg et al., 1980] [Steinberg and Lund, 1989] is semi-empirical. The model is purely empirical and strain-rate independent at high strain-rates. A dislocation-based extension based on [Hoge and Mukherjee, 1977] is used at low strain-rates. The SCGL model is used extensively by the shock physics community. The Zerilli-Armstrong (ZA) model [Zerilli and Armstrong, 1987] is a simple physically based model that has been used extensively. A more

complex model that is based on ideas from dislocation dynamics is the Mechanical Threshold Stress (MTS) model. [Follansbee and Kocks, 1988]. This model has been used to model the plastic deformation of copper, tantalum [Chen and Gray, 1996], alloys of steel [Goto et al., 2000][Banerjee, 2007], and aluminum alloys [Puchi-Cabrera et al., 2001]. However, the MTS model is limited to strain-rates less than around  $10^7 s^{-1}$ . The Preston-Tonks-Wallace (PTW) model [Preston et al., 2003] is also physically based and has a form similar to the MTS model. However, the PTW model has components that can model plastic deformation in the overdriven shock regime (strain-rates greater than  $10^7 s^{-1}$ ). Hence this model is valid for the largest range of strain-rates among the five flow stress models.

The variational framework is compatible with these strain rate and temperature dependent models. Here four flow stress models are described: the Johnson-Cook and Zerilli-Armstrong since they are very classical model, phenomenological power law model [Stainier and Ortiz, 2010] and the Norton-Hoff model [Norton, 1929] [Hoff, 1954] since they may be used in the modeling of the RFW process (see chapter 5).

For each of them, the related stored and dissipated energy are precised. The stored part is obtained by the integration of  $\sigma_y(p_p, \dot{p}_p, T)$  regarding  $p$  and only keeping the terms which do not depend on  $\dot{p}$ . The dissipated part is obtained simply by the integration of  $\sigma_y(p_p, \dot{p}_p, T)$  regarding  $\dot{p}$ .

### 3.1 Power law flow stress model

The power law model used in [Stainier and Ortiz, 2010] represents the material flow as a function of the temperature, the strain and the strain rate. The model we use is composed of a classical power-law term and an exponential saturation term.

$$\sigma_y(\dot{p}, p, T) = \sigma_0(p, T) + \sigma_v(T) \left( \frac{\dot{p}}{\dot{p}_0} \right)^{\frac{1}{m}} \quad (2.70)$$

where

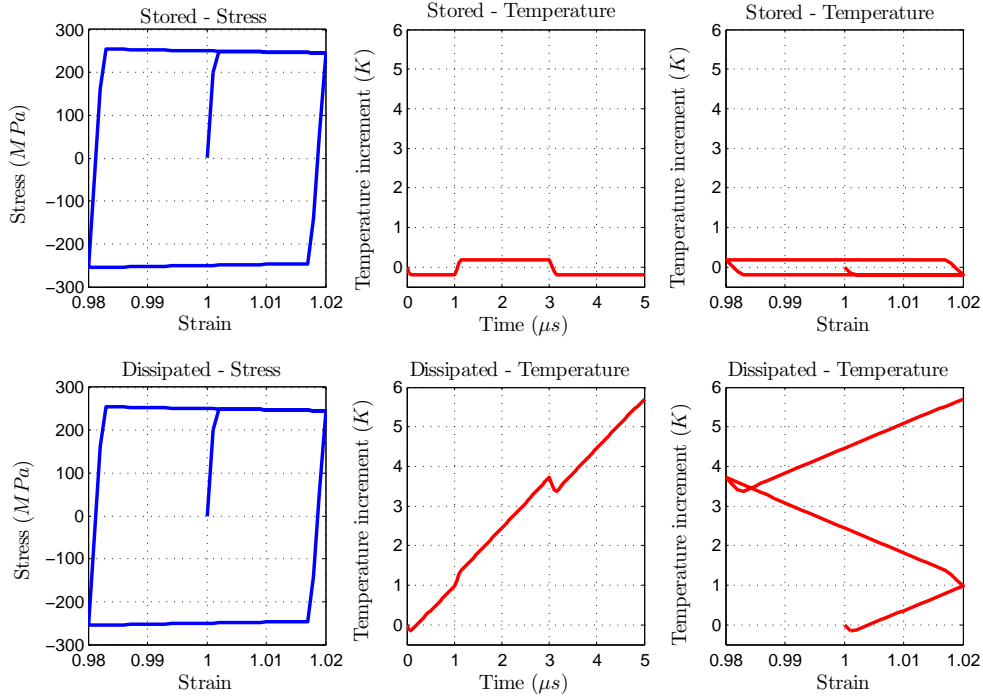
$$\sigma_0(p, T) = \sigma_1(T) (1 + Bp)^{\frac{1}{n}} + \hat{\sigma}_1(T) [1 - \exp(-Dp)] \quad (2.71a)$$

$$\sigma_1(T) = \sigma_1(T_0) [1 - \omega_1(T - T_o)] \quad (2.71b)$$

$$\hat{\sigma}_1(T) = \hat{\sigma}_1(T_0) [1 - \hat{\omega}_1(T - T_o)] \quad (2.71c)$$

$$\sigma_v(T) = \sigma_0(T) [1 - \omega_0(T - T_o)] \quad (2.71d)$$

and where  $B$ ,  $D$ ,  $n$ ,  $n$  and  $\dot{\epsilon}$  are material parameters. The stored and dissipative parts are then given by:



**Figure 2.1:** Influence of the ratio dissipative/stored energy using a power law model.

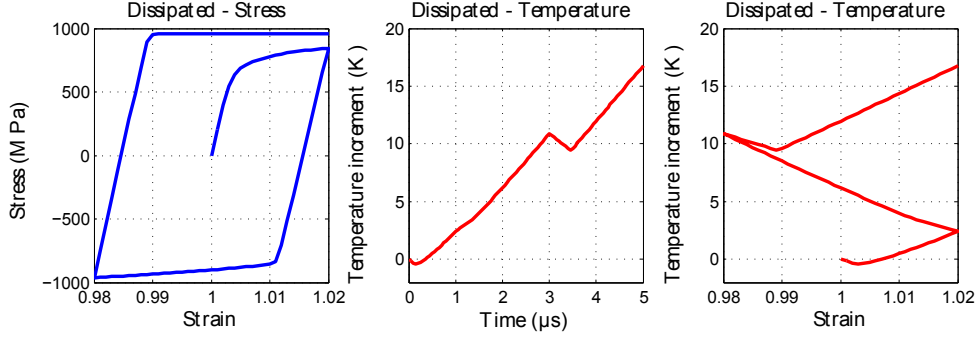
$$W^p(p, T) = \frac{n}{n+1} \frac{\sigma_1(T)}{B} (1 + Bp)^{\frac{1}{n}+1} + \hat{\sigma}_1(T) \left[ p + \frac{1}{D} \exp(-Dp) \right] \quad (2.72)$$

$$\Psi^*(\dot{p}; p, T) = \sigma_0(p, T) \dot{p} + \frac{m}{m+1} \sigma_v(T) \dot{p} \left( \frac{\dot{p}}{\dot{p}_0} \right)^{\frac{1}{m}+1} \quad (2.73)$$

The figure 2.1 shows the material behavior for a maximal stored energy and for a maximal dissipated energy for a uniaxial traction on a mild-steel material. In the two cases, the loading is the same. The evolution of the stress as a function of the strain is the same in both cases. However, the evolution of the temperature inside the material is very different. In the maximal stored case, the temperature is relatively "stable" whereas in the maximal dissipated case, the temperature is greatly increasing.

### 3.2 Norton-Hoff flow stress model

The Norton-Hoff model [Norton, 1929] [Hoff, 1954] represents the material flow as a function of the strain and the strain rate. It is defined by:



**Figure 2.2:** A purely dissipative model: Norton-Hoff model.

$$\begin{aligned}
 \sigma_y(\dot{p}, T) &= K(T) \left( \frac{\dot{p}}{\dot{p}_0} \right)^{1/m} \left( \frac{p}{p_0} \right)^{1/n} \\
 &= A \exp\left(\frac{B}{T}\right) \left( \frac{\dot{p}}{\dot{p}_0} \right)^{1/m} \left( \frac{p}{p_0} \right)^{1/n}
 \end{aligned} \tag{2.74}$$

where  $m$  is the strain-rate dependency exponent,  $n$  the hardening coefficient and  $A$  and  $B$  material constants. This model is purely dissipative and the dissipated energy is given by

$$\Psi^*(\dot{p}; p, T) = \frac{mK}{m+1} \left( \frac{\dot{p}}{\dot{p}_0} \right)^{\frac{1}{m}+1} \left( \frac{p}{p_0} \right)^{\frac{1}{n}} \tag{2.75}$$

The figure 2.2 shows the material behavior for a uniaxial loading on a mild-steel material.

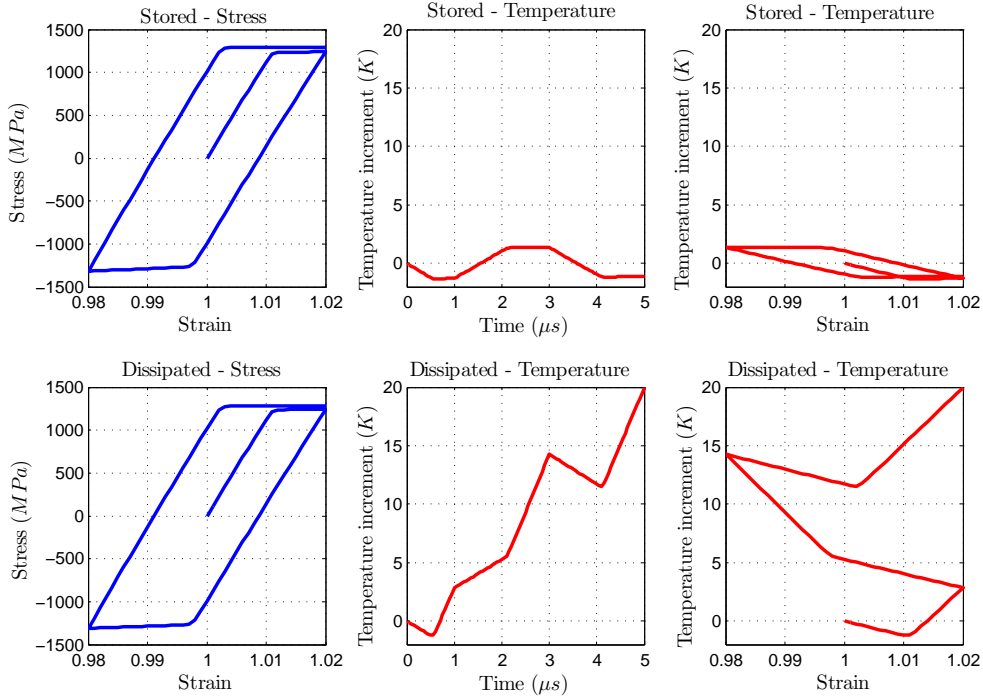
### 3.3 Johnson-Cook flow stress model

Johnson-Cook model [Johnson and Cook, 1983] is considered as the most simple and widely used model to represent the material flow stress as a function of the strain rate, temperature and large equivalent strain. It is empirically defined by

$$\sigma_y(p, \dot{p}, T) = \left[ A + Bp^{\frac{1}{n}} \right] \left[ 1 + C \ln\left(\frac{\dot{p}}{\dot{p}_0}\right) \right] \left[ 1 - (T^*)^{\frac{1}{m}} \right] \tag{2.76}$$

where  $A, B, C, n$  and  $m$  are material constants. The normalized temperature  $T^*$  is defined by

$$T^* = \begin{cases} 0 & \text{if } T < T_0 \\ \frac{T - T_0}{T_m - T_0} & \text{if } T_0 < T < T_m \\ 1 & \text{if } T > T_m \end{cases} \tag{2.77}$$



**Figure 2.3:** Influence of the ratio dissipative/stored energy using Johnson-Cook model.

where  $\dot{p}_0$  is the effective plastic strain rate of the quasi-static test used to determine the yield and hardening parameters  $A, B$  and  $n$ ,  $T_0$  is a reference parameter and  $T_m$  is the melting temperature of the material.

In the variational framework, the stored and the dissipative part of the model should be dissociated. They are respectively defined by

$$W^p(p, T) = \left( A_s p + \frac{B_s}{\frac{1}{n} + 1} p^{\frac{1}{n} + 1} \right) \left( 1 - (T^*)^{\frac{1}{m}} \right) \quad (2.78)$$

$$\Psi^*(\dot{p}; p, T) = \left[ \left( A_d + B_d p^{\frac{1}{n}} \right) \dot{p} + \left( A + B p^{\frac{1}{n}} \right) C \dot{p}_0 \left( \frac{\dot{p}}{\dot{p}_0} \ln \left( \frac{\dot{p}}{\dot{p}_0} \right) - \frac{\dot{p}}{\dot{p}_0} + 1 \right) \right] \left( 1 - (T^*)^{\frac{1}{m}} \right) \quad (2.79)$$

with  $A_s + A_d = A$  and  $B_s + B_d = B$ .

The figure 2.3 shows the material behavior for a maximal stored energy and for a maximal dissipated energy for a uniaxial loading on the titanium Ti-6Al-4V alloy. Again, the curve stress/strain is the same in the two cases but the evolution of the temperature is very different.

### 3.4 Zerilli-Armstrong flow stress model

The Zerilli-Armstrong (ZA) model [Zerilli and Armstrong, 1987] is based on simplified dislocation mechanics. The general form of the equation for the flow stress is

$$\sigma_y(\epsilon_p, \dot{\epsilon}_p, T) = A + Bp^n + (C_1 + C_2\sqrt{p}) \exp(-C_3T) \dot{p}^{cT} \quad (2.80)$$

where  $A$  is a parameter depending on the micro-structure (microstructural stress intensity, average grain diameter, contribution due to solutes and initial dislocation density) and  $B, C_1, C_2$  and  $C_3$  are depending on the type of material: face center cubic (fcc), body-center cubic (bcc), hexagonal close packed (hcp) or alloys. As for JC flow stress model, the stored and dissipated part of the model should be dissociated. They are respectively defined by

$$W^p(p, T) = \left( A_s p + \frac{B_s}{n+1} p^{n+1} \right) \quad (2.81)$$

$$\Psi^*(\dot{p}; p, T) = (A_d + B_d p^n) \dot{p} + (C_1 + C_2 \sqrt{p}) \frac{\exp(-C_3 T)}{cT + 1} \dot{p}^{cT+1} \quad (2.82)$$

For instance, a fcc material implies that  $B_s = B_d = 0$  and  $C_1 = 0$  whereas a bcc material implies that  $C_2 = 0$ .

## 4 Conclusion

A variational framework has been implemented. An incremental approach has been used so that the continuous formulation has successively been temporally and spatially discretized. The spatial discretization has been made regarding that a meshless method and material points integration are going to be used. The fact that the shape functions are defined over a domain larger than the FEM elements is used in equation 2.69.

Now that this framework is set, we are going to construct the LME interpolation in the next chapter. In this chapter, the LME shape functions will to be constructed.



## Chapter 3

# Local Maximum Entropy interpolation

*In this chapter, the theory of maximum entropy interpolation is presented. From its historical origins to the shape functions expressions, its main characteristics will be explained and demonstrated. The principle of the method is quite simple: from data given on a cloud of points, find the optimal interpolation functions in the sense of the information theory by maximizing the entropy and then use and adapt these functions in a meshless method.*



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# 1 Information theory

The information theory has been developed by Claude E. Shannon [Shannon, 1948]. It is a probabilistic theory used to quantify the information contained in a given data set. One of the characteristics of this theory is that it gives an entirely physical status to the information notion: left to itself, information can only go in the direction of its disorganization, that is to say, the entropy increase. When an information suffers a transformation such as transmission or coding, it also suffers an inevitable and increasing degradation. This is how Shannon defines the entropy as a measure of the information:

$$H(p) = -p \log(p) \quad (3.1)$$

where  $p$  is the probability related to a given event.

This is very close to the entropy  $\eta$  defined by Boltzmann-Gibbs in thermo-dynamics and used into the previous chapter (even if it is a coincidence since there is no direct "relationship" between those two).

Therefore, the entropy  $H$  of the set  $X$  is defined for a set  $X$  of  $n$  events  $i$  associated to the probabilities  $p_i$ . The probabilistic distribution which is maximizing  $H$  is the most *natural* and the least-biased possible.

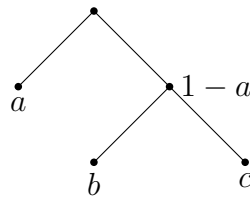
**Theorem 3.1** ([Shannon, 1948]). Consider  $A_1, \dots, A_n$   $n$  events associated to the probabilities  $p_1, \dots, p_n$ . Let  $H(p_1, \dots, p_n)$  the entropy, a measure of the uncertainty related to the events  $A_1, \dots, A_n$ .

Therefore,  $H : \mathbb{R}^n \mapsto \mathbb{R}$  must verify:

- $H$  is positive and must be continuous in each of its variables,
- if  $\forall a \in [1, n], p_a = \frac{1}{n}$ , then  $H$  must be monotone and growing when  $n$  is increasing,
- if there are numerous levels of choice, then  $H$  is the sum of the sub- $H$ .

Example: Let us consider  $a, b$  and  $c$  positive such that  $a + b + c = 1$ .

Then  $H(a, b, c) = H(a, 1 - a) + (1 - a)H(\frac{b}{b+c}, \frac{c}{b+c})$



The only functions  $H$  verifying those three conditions are:

$$H(p_1, \dots, p_n) = -K \sum_{i=1}^n p_i \log(p_i)$$

with  $K \in \mathbb{R}_+^*$ .

**Proof.** Let  $P_1, \dots, P_n$   $n$  equiprobable events linked to the probabilities  $p_1, \dots, p_n$ . Therefore,  $\exists A \in \mathcal{C}^0(\mathbb{R}, \mathbb{R}) : A(n) = H(\frac{1}{n}, \dots, \frac{1}{n})$ . The third conditions gives  $\forall (s, t, m, n) \in \mathbb{N}_+^{*4}$ ,  $A(s^m) = mA(s)$  and  $A(t^n) = nA(t)$ .  $\forall n$  large enough,  $\exists m : s^m \leq t^n \leq s^{m+1}$ .

– Because the log function is increasing and  $\log(s) > 0$  :

$$\left| \frac{\log(t)}{\log(s)} - \frac{m}{n} \right| \leq \frac{1}{n} = \epsilon \quad (3.2)$$

– According to the second condition,  $A$  is monotone. Therefore:

$$A(s^m) \leq A(t^n) \leq A(s^{m+1}).$$

By using the first condition:

$$\left| \frac{m}{n} - \frac{A(t)}{A(s)} \right| \leq \frac{1}{n} = \epsilon \quad (3.3)$$

Equations 3.2 and 3.3 result in

$$\left| \frac{A(t)}{A(s)} - \frac{\log(t)}{\log(s)} \right| \leq 2\epsilon \quad (3.4)$$

This is true  $\forall (s, t)$  and  $A$  is increasing therefore

$$K \in \mathbb{R}_+^* : A(n) = K \log\left(\frac{1}{n}\right) \quad (3.5)$$

Now, consider a first case of  $n$  events  $B_1, \dots, B_n$  associated to probabilities  $q_1, \dots, q_n$  such as  $\forall a \in \llbracket 1, n \rrbracket$ ,  $q_a = \frac{n_a}{\sum_{j=1}^n n_j}$  and for each event  $B_a$  there are  $n_a$  equiprobable "sub-events".

Then this first case is equivalent to the second case where  $\sum_{j=1}^n n_j$  equiprobable events.

$$H(\text{case 2}) = K \log\left(\sum n_a\right) \quad (3.6)$$

and according to the third condition,

$$H(\text{case 1}) = H(q_1, \dots, q_n) + K \sum q_a \log(n_a) \quad (3.7)$$

Therefore

$$H(q_1, \dots, q_n) + K \sum q_a \log(n_a) = K \log(\sum n_a) \quad (3.8)$$

And finally:

$$H(q_1, \dots, q_n) = -K \sum_{a=1}^n q_a \log q_a \quad (3.9)$$

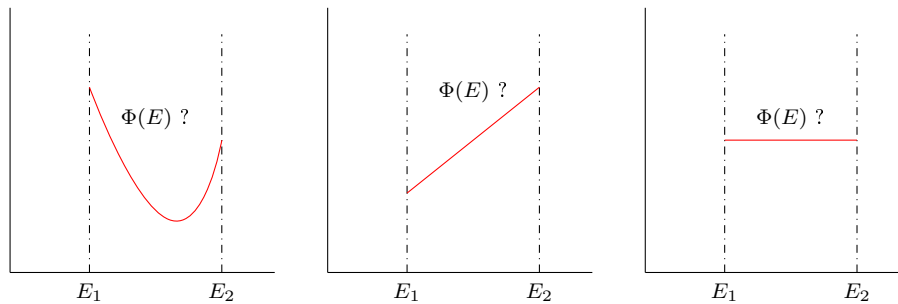
□

**Example 3.1.** Consider a set  $X$  of two events  $A_1$  and  $A_2$  with the two following probability distributions:

$$X_1 = \begin{pmatrix} A_1 & A_2 \\ 0.5 & 0.5 \end{pmatrix} \text{ and } X_2 = \begin{pmatrix} A_1 & A_2 \\ 0.8 & 0.2 \end{pmatrix}$$

The result of the first case scenario is more uncertain than the second one:  $H(X_1) > H(X_2)$ .

**Example 3.2.** Consider a more physical example. Suppose that the value of a Young modulus  $E$  must be known. All that is known about it is that  $E \in [E_1, E_2]$ . How must the probability density law be chosen to approximate  $E$  a priori without more information ?



**Figure 3.1:** Choice of probability density law a priori ?

Shannon's maximum entropy principle stipulates that the good choice is the last displayed on figure 3.1 because it maximizes the entropy.

## 2 Local Maximum Entropy problem

### 2.1 Notations

Consider the following node set  $X$ :

$$X = \{x_a, a = 1, \dots, n\} \subset \mathbb{R}^d$$

The smallest convex space containing  $X$  is given by [Rockafellar, 1997]:

$$\text{conv}X = \{\mathbf{x} \in \mathbb{R}^d | \exists \Phi \in \mathbb{R}_+^n : \Phi \cdot \mathbf{1} = 1 \text{ and } \mathbf{x} = \Phi \cdot \mathbf{X}\}$$

where:

- $\mathbb{R}_+^n$  is the positive orthant
- $\mathbf{1} \in \mathbb{R}^n$  is a vector full of 1
- $\mathbf{X}$  is a matrix of  $d$  lines and  $N$  columns in which the columns are the coordinates of nodes of  $X$

Since  $X$  is finite,  $\text{conv}X$  is convex and compact. Let  $u : \text{conv}X \mapsto \mathbb{R}$ . Assume that  $\{u_a = u(x_a) | a = 1, \dots, n\}$  is known on  $X$ . An approximation of  $u$  is built as following:

$$u^h(\mathbf{x}) = \sum_{a=1}^n p_a(\mathbf{x}) u_a \quad (3.10)$$

where  $p_a : \text{conv}X \rightarrow \mathbb{R}$  are called the shape or interpolation functions. Those functions must verify the following conditions:

$$\sum_{a=1}^n p_a(\mathbf{x}) = 1 \quad (3.11a)$$

$$\sum_{a=1}^n p_a(\mathbf{x}) \mathbf{x}_a = \mathbf{x} \quad (3.11b)$$

## 2.2 Maximum Entropy basis functions

This section is based on the paper [Arroyo and Ortiz, 2006]. Most of the content can be found in this article.

A link has been established between the Shannon entropy and the building of shape functions for a given node set [Arroyo and Ortiz, 2006]. First of all, one does want to use the least-biased probability distribution according to Jaynes' principle [Jaynes, 1957]. Therefore, for a given node set, the solution  $p = [p_1, \dots, p_n]$  of the problem must be found:

$$\text{(ME) Maximize} \quad H(\mathbf{p}) = - \sum_{a=1}^n p_a \log p_a \quad (3.12)$$

$$\text{such that } \forall \mathbf{x} \in \text{conv}X \quad p_a(\mathbf{x}) \geq 0, a = 1, \dots, N$$

$$\sum p_a(\mathbf{x}) = 1$$

$$\sum p_a(\mathbf{x}) \mathbf{x}_a = \mathbf{x}$$

The problem 3.12 has a solution if, and only if,  $\mathbf{x} \in \text{conv}X$ . Then, this solution is unique. The existence and unicity of the solution are proven using the convexity properties of  $\text{conv}X$  and the strict convexity of the function  $-H$  [Arroyo and Ortiz, 2006].

These functions, even if they are optimal in an information-theoretical sense, are non-local and not very efficient as shape functions in mechanical modeling. To increase their efficiency, it is necessary to add a notion of locality: concretely, what happens near the considered point  $x$  is the most interesting. This notion of locality can be linked to the width of the functions  $p_a$  [Arroyo and Ortiz, 2006]:

$$\begin{aligned} \text{(RAJ) Minimize} \quad U(\mathbf{x}, \mathbf{p}) &\equiv \sum_{a=1}^n p_a |\mathbf{x} - \mathbf{x}_a|^2 \\ \text{such as } \forall \mathbf{x} \in \text{conv}X \quad p_a(\mathbf{x}) &\geq 0, a = 1, \dots, N \\ &\sum p_a(\mathbf{x}) = 1 \\ &\sum p_a(\mathbf{x}) \mathbf{x}_a = \mathbf{x} \end{aligned} \tag{3.13}$$

As for problem 3.12, the existence of solutions for the problem 3.13 can be proved. The difference is that  $U(\mathbf{x}, \cdot)$  is linear, i.e. not strictly convex, so in general, there is no unicity of the solution. Nevertheless, there is unicity of the solution if the nodes are in general position [Rajan, 1994] and this solution is the Delaunay approximation scheme.

Thus far, two criteria for selecting a convex approximation scheme have been defined: the maximum entropy and the maximum locality. In general, it is not possible to find convex approximations schemes which are solution of the both problems. The classical way to harmonize such competitive objectives is to find a Pareto optima, i.e. the *best* compromise solution.

**Definition 3.1.** *A convex approximation scheme  $q$  is better than, or dominates,  $p$  if, and only if,  $-H(q) \geq -H(p)$ ,  $U(x, q) \geq U(x, p)$  and at least one of the inequalities is strict.*

A way to find the Pareto set is to scalarize the problem. Thus, the following Local Maximum Entropy problem [Arroyo and Ortiz, 2006] must be solved:

$$\begin{aligned} \text{(LME)}_\beta \text{ Minimize} \quad f_\beta(\mathbf{x}, \mathbf{p}) &\equiv \beta U(\mathbf{x}, \mathbf{p}) - H(\mathbf{p}) \\ \text{such that } \forall \mathbf{x} \in \text{conv}X \quad p_a(\mathbf{x}) &\geq 0, a = 1, \dots, N \\ &\sum p_a(\mathbf{x}) = 1 \\ &\sum p_a(\mathbf{x}) \mathbf{x}_a = \mathbf{x} \end{aligned} \tag{3.14}$$

For  $\beta \in ]0; +\infty[$ , the solution of the problem  $(\text{LME})_\beta$  3.14 is Pareto optimal. For  $\beta = 0$ , the solution is the unique solution of the problem (ME) 3.12. For  $\beta = +\infty$ , solutions are the solutions of the problem (RAJ) 3.13.

**Proposition 3.1** ([Arroyo and Ortiz, 2006]). Consider a known node set  $X$ . Let  $\mathbf{x} \in \text{int}(\text{conv}X)$  and  $\beta \in [0; +\infty[$ .

The unique solution of the problem  $(LME)_\beta$  3.14 is

$$p_{\beta a}(\mathbf{x}) = \frac{1}{Z(\mathbf{x}, \boldsymbol{\lambda}^*(\mathbf{x}))} \exp[-\beta|\mathbf{x} - \mathbf{x}_a|^2 + \boldsymbol{\lambda}^* \cdot (\mathbf{x} - \mathbf{x}_a)] \quad (3.15)$$

where:

$$\boldsymbol{\lambda}^*(\mathbf{x}) = \arg \min_{\boldsymbol{\lambda} \in \mathbb{R}^d} \log Z(\mathbf{x}, \boldsymbol{\lambda}) \quad (3.16)$$

$$Z(\mathbf{x}, \boldsymbol{\lambda}) = \sum_{a=1, \dots, n} \exp[-\beta|\mathbf{x} - \mathbf{x}_a|^2 + \boldsymbol{\lambda} \cdot (\mathbf{x} - \mathbf{x}_a)] \quad (3.17)$$

Furthermore, the minimizer  $\boldsymbol{\lambda}^*(\mathbf{x})$  is unique.

The whole proof is available in [Arroyo and Ortiz, 2006]. In practice, dimensionless parameter  $\gamma$  is used. It is related to  $\beta$  by

$$\gamma = h^2 \beta \quad (3.18)$$

where  $h$  is a local characteristic length of the considered node set  $X$ . Hence, the shape functions become

$$p_{\beta a}(\mathbf{x}) = \frac{1}{Z(\mathbf{x}, \boldsymbol{\lambda}^*(\mathbf{x}))} \exp\left[-\frac{\gamma}{h^2}|\mathbf{x} - \mathbf{x}_a|^2 + \frac{\boldsymbol{\lambda}^*}{h}(\mathbf{x} - \mathbf{x}_a)\right] \quad (3.19)$$

where

$$\boldsymbol{\lambda}^*(\mathbf{x}) = \arg \min_{\boldsymbol{\lambda} \in \mathbb{R}^d} \log \left( \sum_{a=1}^n \exp\left[-\frac{\gamma}{h^2}|\mathbf{x} - \mathbf{x}_a|^2 + \frac{\boldsymbol{\lambda}}{h}(\mathbf{x} - \mathbf{x}_a)\right] \right) \quad (3.20)$$

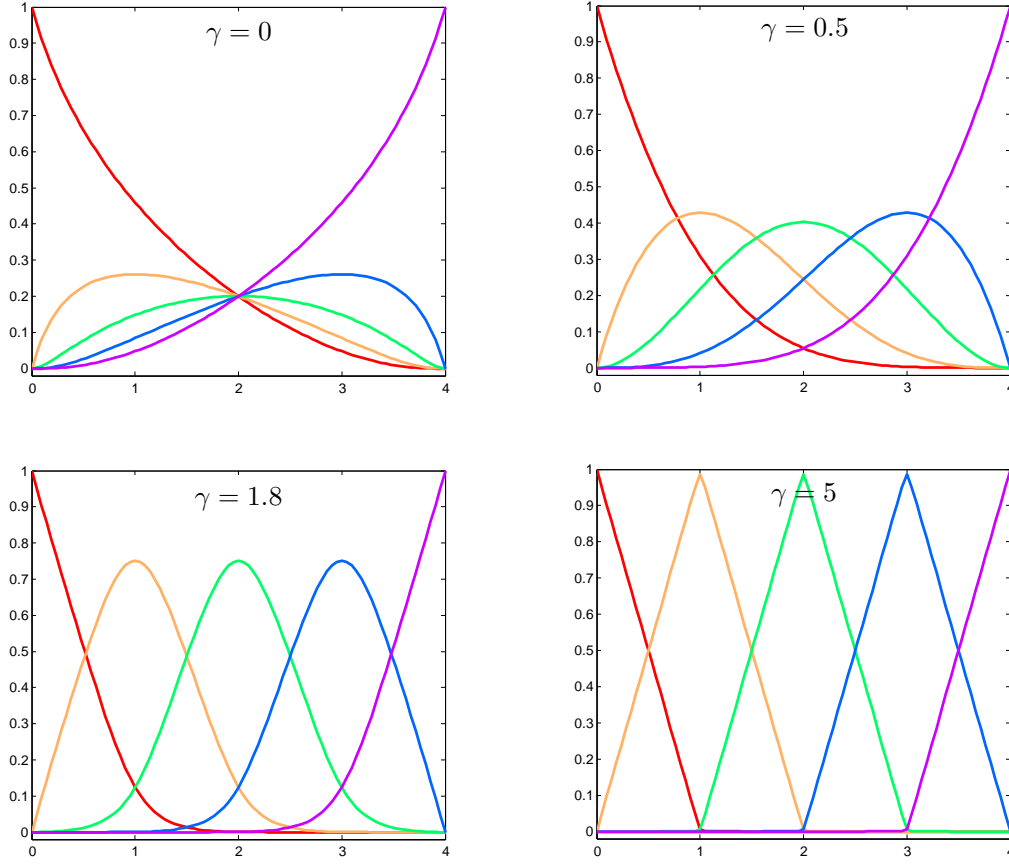
The apparition of the term  $\frac{\boldsymbol{\lambda}^*}{h}$  instead of  $\boldsymbol{\lambda}^*$  is used to scale relatively to  $h$  and to help the convergence for the computation of  $\boldsymbol{\lambda}^*$ . Moreover, the obtained values of  $p_{\beta a}(\mathbf{x})$  are more accurate when this term is added.

Figure 3.2 represents the shape function for an initial node set in 1D for different values of  $\gamma$ .

## 3 Some characteristics of the maximum entropy interpolation

### 3.1 Dirichlet boundary condition

As announced in the introduction, one of the major advantages of the maximum entropy interpolation is that it allows Dirichlet boundary conditions, which is absolutely necessary for the RFW modeling.



**Figure 3.2:** 1D MaxEnt shape functions depending on  $\gamma$ .

**Property 3.1** ([Arroyo and Ortiz, 2006]). Consider  $p = p_1, \dots, p_n$  a convex approximation scheme,  $X = x_1, \dots, x_n$  a known node set and  $\delta conv X$  the boundary of  $conv X$ .

Let  $x \in \delta conv X \setminus X$ . Then,

$$\forall a \in [1, n], x_a \in conv X \setminus \delta conv X \Rightarrow p_a(x) = 0.$$

**Proof.** Ad absurdum, suppose that  $\exists a \in [1, n]$  such as  $x_a \in conv X \setminus \delta conv X$  and  $p_a(x) \neq 0$ . Therefore,

$$x = \sum_{b \neq a} p_b(x) x_b + p_a(x) x_a$$

$p_a(x) \neq 0$  implies that  $\sum_{b \neq a} p_b(x) \neq 0$  because  $x \notin X$ . Now consider  $f : [0, 1] \mapsto conv X$  such as

$$\forall t \in [0, 1], f(t) = t \frac{\sum_{b \neq a} p_b(x) x_b}{\sum_{b \neq a} p_b(x)} + (1 - t) x_a$$



Then,  $f(1 - p_a(x)) = x$  because of equation 3.11a. It follows that  $x$  is in the segment  $\left[ \frac{\sum_{b \neq a} p_b(x)x_b}{\sum_{b \neq a} p_b(x)}, x_a \right]$ . As  $x \in \delta conv X$ , either  $\frac{\sum_{b \neq a} p_b(x)x_b}{\sum_{b \neq a} p_b(x)}$  or  $x_a$  must be in  $\delta conv X$ .  $\frac{\sum_{b \neq a} p_b(x)x_b}{\sum_{b \neq a} p_b(x)} \notin \delta X$  so  $x_a \in \delta X$ , which is a contradiction with the first hypothesis. □

As the functions are exponential, it is important to note that they are never equal to zero on  $conv X$ . However, they are small enough on  $\delta conv X$  to be considered null (between  $10^{-7}$  and the machine precision in the example of figure 3.2).

### 3.2 Positivity of the Jacobian in $\lambda^*$ computation

Let  $x \in \text{int}(conv X)$  and  $\lambda \in \mathbb{R}^n$ . The Jacobian in  $\lambda^*$  computation is the Hessian of the function  $\log Z(\mathbf{x}, \lambda)$ .

$$\mathbf{J}(\mathbf{x}, \lambda) \equiv \partial_\lambda \partial_\lambda \log Z(\mathbf{x}, \lambda) = \sum_{a=1}^N p_a(\mathbf{x}, \lambda) (\mathbf{x} - \mathbf{x}_a) \otimes (\mathbf{x} - \mathbf{x}_a) - r(\mathbf{x}, \lambda) \otimes r(\mathbf{x}, \lambda)$$

with

$$r(\mathbf{x}, \lambda) = \partial_\lambda \log Z(\mathbf{x}, \lambda) = \sum_{a=1}^N p_a(\mathbf{x}, \lambda) (\mathbf{x} - \mathbf{x}_a)$$

Let  $\mathbf{u} \in \mathbb{R}^{*n}$  and  $u_a = \mathbf{u}(\mathbf{x} - \mathbf{x}_a)$ . Hence, by convexity of the square function:

$$\mathbf{u} \cdot \mathbf{J}(\mathbf{x}, \lambda) \cdot \mathbf{u} = \sum_{a=1}^N p_a(\mathbf{x}, \lambda) u_a^2 - \left( \sum_{a=1}^N p_a(\mathbf{x}, \lambda) u_a \right)^2 > 0$$

It results that the Jacobian is symmetric positive definite.

### 3.3 Link with numerical modeling

As presented in the introduction of this thesis, a particularity of our maximum entropy approach is that we introduce material points in addition to nodes. Here are the necessary components for the thermo-mechanical modeling.

The nodes are as necessary as in any other method. The information is stored at the nodes: displacements in mechanics, temperature in thermics and both in thermo-mechanics.

The material points are used for integration (see chapter 2), just like the Gauss points do in classical FEM. Inner variables such as plastic strains, or temperature in adiabatic computation, and material data such as Young modulus or Poisson

coefficient are stored at the material points. The position of the material point will be discussed on the following section.

A notion of neighborhood has also been added. Even if the shape functions are defined over  $\text{conv}X$ , and if  $\beta$  is large enough, the locality term imply that if  $x \in \text{conv}X$  and  $x_a \in X$  are *far enough* from each other, then  $p_a(x) < \epsilon$  where  $\epsilon$  is *small* (typically  $\epsilon < 10^{-12}$ ). Without neighborhoods, all  $\text{conv}X$  is considered for each shape function and the result is full matrices with a certain number of *small* terms. To avoid this,  $\text{conv}X$  is cut and a neighborhood, i.e. a set of nodes close enough of the considered material point, is considered instead.

## 4 Implementation choices

A few choices have been made concerning the implementation of the maximum entropy method. This section will describe the main features.

### 4.1 Regularized Newton method

In order to reduce the number of iteration in the Newton-Raphson algorithm to determine  $\boldsymbol{\lambda}^*(\mathbf{x}) = \arg \min_{\boldsymbol{\lambda} \in \mathbb{R}^d} \log Z(\mathbf{x}, \boldsymbol{\lambda})$ , the algorithm has been modified [Polyak, 2009]. Let define:

$$F(\mathbf{x}, \boldsymbol{\lambda}, \mathbf{y}) = \log Z(\mathbf{x}, \mathbf{y}) + \frac{1}{2} \|\nabla \log Z(\mathbf{x}, \boldsymbol{\lambda})\| \|\mathbf{y} - \boldsymbol{\lambda}\|^2 \quad (3.21)$$

It follows that  $\boldsymbol{\lambda}^*(\mathbf{x}) = \arg \min_{\boldsymbol{\lambda} \in \mathbb{R}^d} F(\mathbf{x}, \boldsymbol{\lambda}, \mathbf{y})$  with  $\boldsymbol{\lambda} = \mathbf{y}$ . Indeed:

- $F(\mathbf{x}, \boldsymbol{\lambda}, \mathbf{y})|_{\mathbf{y}=\boldsymbol{\lambda}} = \log Z(\mathbf{x}, \boldsymbol{\lambda})$
- $\nabla_{\mathbf{y}} F(\mathbf{x}, \boldsymbol{\lambda}, \mathbf{y})|_{\mathbf{y}=\boldsymbol{\lambda}} = \nabla \log Z(\mathbf{x}, \boldsymbol{\lambda})$
- $\nabla_{\mathbf{y}\mathbf{y}} F(\mathbf{x}, \boldsymbol{\lambda}, \mathbf{y})|_{\mathbf{y}=\boldsymbol{\lambda}} = \nabla^2 \log Z(\mathbf{x}, \boldsymbol{\lambda}) + \|\nabla \log Z(\mathbf{x}, \boldsymbol{\lambda})\| I$  where  $I$  is the identity matrix of  $\mathbb{R}^{\dim}$ .

In the algorithm, we set  $\mathbf{y} = \boldsymbol{\lambda}^{(k)}$  to find  $\boldsymbol{\lambda}^{(k+1)}$ . The additional term in the Hessian solves the issues of singular matrices and allows faster convergence. Simple tests in cases where  $\gamma$  and the density of node are large have proved the efficiency of this regularization: the classical Newton-Raphson algorithm needed more than forty iterations while the regularized one only needed less than ten.

### 4.2 Mesh use

In order to position the material points, it is assumed that their distribution over  $\text{conv}X$  is similar to the nodes distribution. An idea is to use a mesh generated by a third software (Gmsh, Abaqus, etc.). Having a list of simplices (elements)

facilitates the creation of the material points in the program. By doing a loop over the simplices, the material points are created and distributed relatively to the nodes at positions of Gauss points for example.

It is important to note that the mesh and the simplices are only used to create the material points. Once it is done, they are no longer needed nor used.

### 4.3 Choice of the characteristic length $h$

As indicated previously, the dimensionless parameter  $\gamma = h^2\beta$  is used to adjust the shape functions. The manipulation of a dimensionless parameter is more convenient but it implies the determination of the parameter  $h$ , which dimension is a length.  $h$  is chosen to be proper to each material point and their associated simplex. Hence  $h$  is arbitrary defined:

- $h$  is defined for each material point;
- $h$  is calculated only for simplices containing material points and is depending on the dimension:
  - In one dimension, a simplex is a segment of length  $l$ :  $h = l$ .
  - In two dimensions, a simplex is a triangle of area  $A$ :  $h = \sqrt{A}$ .
  - In three dimensions, a simplex is a tetrahedron of volume  $V$ :  $h = \sqrt[3]{V}$

This choice of  $h$  is relevant in case of traction. Indeed, choosing  $h$  as a constant may lead to non physical rupture of the material in traction tests. On the contrary for compressive tests, a constant  $h$  parameter should work with updates of  $h$ .

This choice of  $h$  is also used in the way the neighborhoods of each material point are chosen. Indeed, a node  $x_a$  is in the neighborhood of material point  $x_{mPt}$  if, and only if:

$$\|x_{mPt} - x_a\| < h\sqrt{\frac{\text{TOL}}{\gamma}} \quad (3.22)$$

where TOL=  $10^{-16}$  by default. This condition allows to have not completely full matrices.

## 5 Material points

Now that the expression of the shape functions has been established, it must be used to model the thermo-mechanical problems presented in the previous chapter. The material points are used for integration over their neighborhood. The question is:

where must they be put? In theory, they can be put everywhere. But there might be some positions better than others.

Basically, the material points are the only positions where the shape functions are computed. It means that for each material point and each shape function, the corresponding  $\boldsymbol{\lambda}^*$  must be computed. An idea is to find, for a given node set  $X$ , the location where  $\boldsymbol{\lambda}^* = \mathbf{0}$ . The motivation is that the constraints of problem 3.14 are naturally verified at  $\boldsymbol{\lambda}^* = \mathbf{0}$ . The objective is to find a set  $M$  defined by

$$M = \{\boldsymbol{x} \in \text{conv}X \setminus \delta\text{conv}X \mid \boldsymbol{\lambda}(\boldsymbol{x}) = \mathbf{0}\} \quad (3.23)$$

## 5.1 Optimized position for material points

### 5.1.1 Mathematical problem

The Lagrangian multiplier  $\boldsymbol{\lambda}$  is used to insert the conditions of problems  $(LME)_\beta$  3.14 in the shape functions. Therefore, if at some point the Lagrangian multipliers are equal to zero, it means that the conditions of  $(LME)_\beta$  3.14 are naturally satisfied. The idea is to put the integration points at those positions in order to simplify the shape functions expression. Moreover in a full Lagrangian point of view, if the whole set  $M$  is found,  $\boldsymbol{\lambda}^*$  no more have to be computed since it will always be equal to zero during the computation as the shape functions are only computed on the material point.

The determination of the set  $M$  is based on the expression of  $\boldsymbol{\lambda}^*(\boldsymbol{x})$ . A point  $x \in M$  if

$$\boldsymbol{\lambda}^*(\boldsymbol{x}) = \arg \min_{\boldsymbol{\lambda} \in \mathbb{R}^d} \log Z(\boldsymbol{x}, \boldsymbol{\lambda}) = \mathbf{0} \quad (3.24)$$

$M$  is exactly the solution of 3.24. Since the minimizer  $\boldsymbol{\lambda}^*(\boldsymbol{x})$  is unique,  $M$  is also the exact solution of system 3.25.

$$\frac{\partial \log Z}{\partial \boldsymbol{\lambda}}(\boldsymbol{x}, \mathbf{0}) = 0 \quad (3.25a)$$

$$\frac{\partial^2 \log Z}{\partial \boldsymbol{\lambda}^2}(\boldsymbol{x}, \mathbf{0}) \geq 0 \quad (3.25b)$$

Let the functions  $\psi$ ,  $\psi_1$  and  $\psi_2$  defined by

$$\psi = \frac{\partial \log Z}{\partial \boldsymbol{\lambda}} \quad (3.26)$$

$$\psi_1 = \frac{\partial \log Z}{\partial \lambda_1} \quad (3.27)$$

$$\psi_2 = \frac{\partial \log Z}{\partial \lambda_2} \quad (3.28)$$

In 1D, equations 3.25 become:

$$\sum_{a=1}^N (x - x_a) e^{-\beta(x-x_a)^2} = 0 \quad (3.29)$$

In 2D, equations 3.25 become:

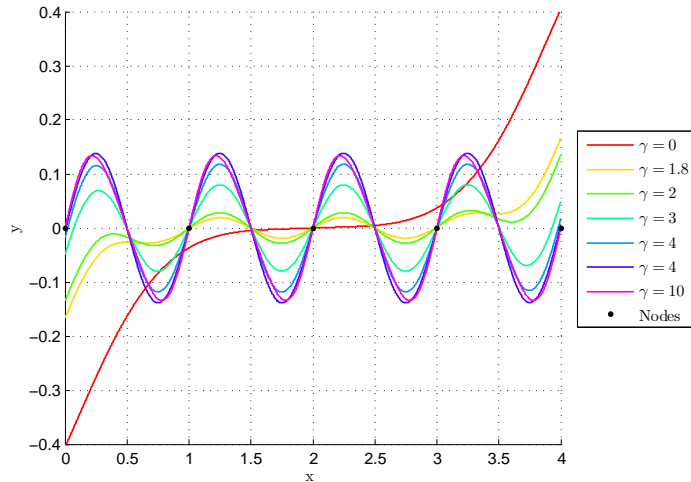
$$\sum_{a=1}^N (x - x_a) e^{-\beta((x-x_a)^2 + (y-y_a)^2)} = 0 \quad (3.30a)$$

$$\sum_{a=1}^N (y - y_a) e^{-\beta((x-x_a)^2 + (y-y_a)^2)} = 0 \quad (3.30b)$$

An important point to notice is that material points have to be put where  $\lambda = \mathbf{0}$  but not everywhere  $\lambda = \mathbf{0}$ . Indeed, so many material points are not needed to integrate the shape functions all over the domain correctly so all the solutions of 3.24 are not necessarily needed.

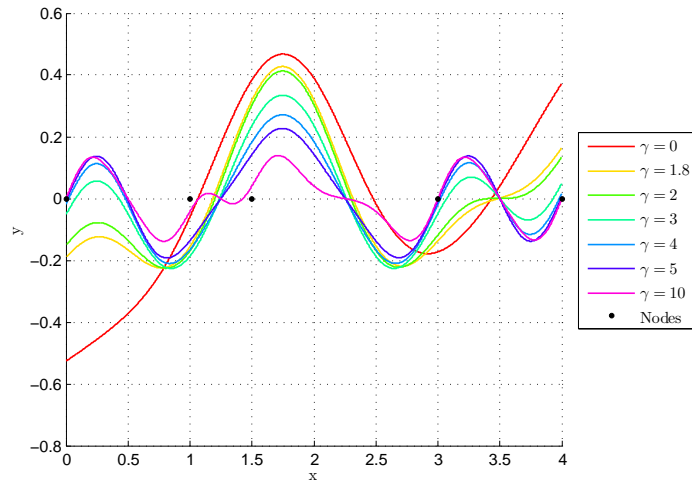
### 5.1.2 Study in one dimension

$M$  is highly dependent of the initial node-set  $X$  and  $\gamma$ . Figures 3.3 and 3.4 show the influence of the position of one single node for a given node set over the function  $\lambda(x)$  in one dimension. In the first case, we consider five nodes regularly disposed. In the second case, one node has been moved.



**Figure 3.3:** Example 1:  $\psi(x)$  in a 1D model with regularly spaced nodes.

The number of solutions  $x$  such that  $\psi(x) = 0$  depends on the value of  $\gamma$  but also on the nodes positions and it seems that there is no obvious rule. Because the number



**Figure 3.4:** Example 2:  $\psi(x)$  in a 1D model with an irregularity.

Example	$\gamma$	Number of solutions
1	1.0	1
2	1.0	3
1	1.8	5
2	1.8	3
1	3.0	9
2	3.0	7
1	5.0	9
2	5.0	7

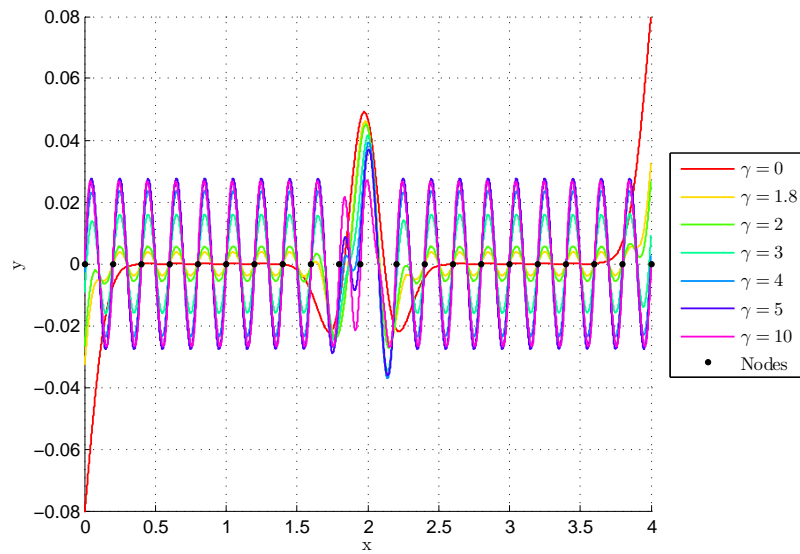
**Table 3.1:** Comparison of the number of solution in 1D depending on the node-set and the value of  $\gamma$ .

of solution is unknown a priori, looking for every solutions by a Newton-Raphson algorithm is in general ineffective since it is impossible to know if every solutions have been found.

However, since the function 3.29 is very smooth, all the solutions can be found by starting a Newton-Raphson algorithm from different initial values (typically a hundred starting values all over the  $convX$ ).

An other problematic aspect is that the behavior of  $\psi$  highly depends on the regularity of the node-set over the considered neighborhood. The figure 3.5 shows  $\psi$  for a dense node-set with only one irregularity. In this case,  $\Delta x = h = 0.2$  and the coordinate of the tenth node is 1.95 instead of 2.

It appears that  $\psi$  has a regular behavior as soon as the node-set is regular all over the domain. For this reason, a node-set as regular as possible is used.

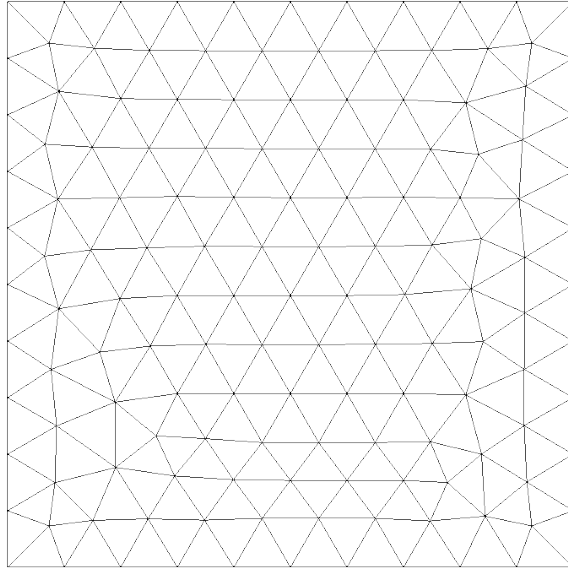


**Figure 3.5:** Example 3:  $\psi(x)$  in a 1D model for a dense node-set with a small irregularity.

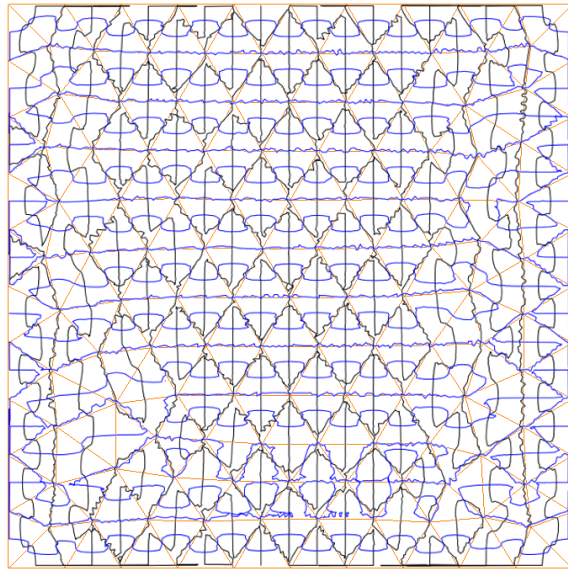
### 5.1.3 Study in two dimension

At first, a simple geometry and a node-set, as regular as possible in order to avoid the irregularity problems, are used. Figure 3.6 shows the initial mesh generated by Gmsh. In general,  $\gamma$  is locally adapted to the node-set by the creation of neighborhood where the node-set is *regular*.

From this node-set,  $\psi_1$  and  $\psi_2$  can be evaluated at different coordinates and then a Delaunay interpolation can be used to find out the iso-0 of the two functions.



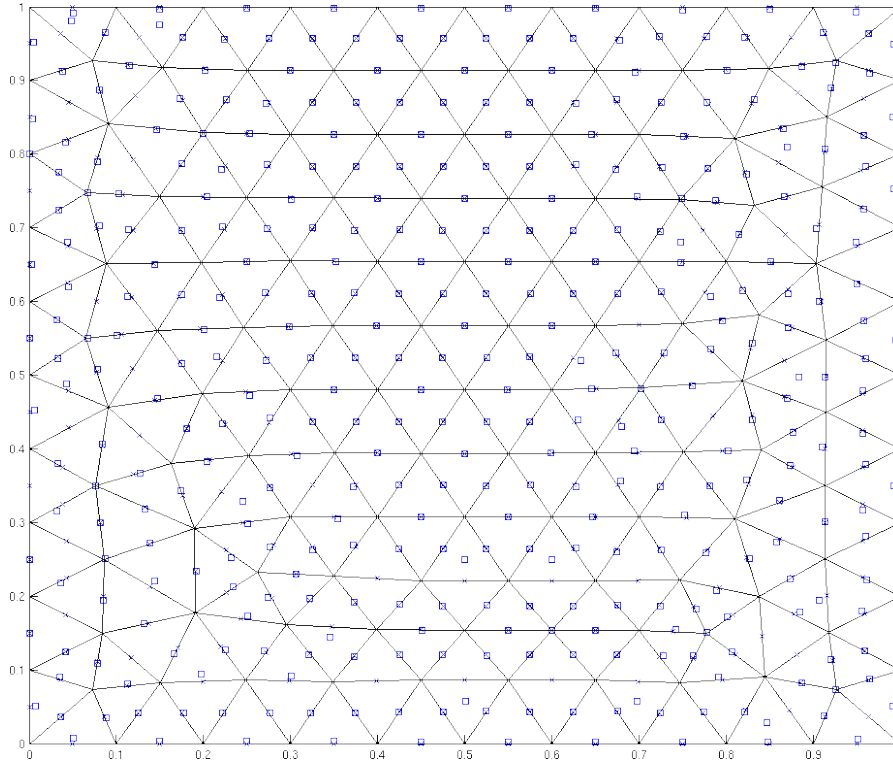
**Figure 3.6:** Initial grid for the determination of the material points.



**Figure 3.7:** Iso-0 of  $\psi_1$  and  $\psi_2$ .



The solutions of the system 3.30 is the set formed by  $\phi = \{iso0(\psi_1)\} \cap \{iso0(\psi_2)\}$ . Figure 3.7 shows that both of the iso-0 are quite regular where the *mesh* is regular (mostly at the center of the domain *convX*). On the contrary, there are some irregularities around the corners. It appears that  $\phi$  is mainly composed of middle points of every single edge of the *mesh*. The strategy is to begin a Newton-Raphson algorithm at those coordinates in order to find the nearest solution. Thus figure 3.8 is obtained.



**Figure 3.8:** Optimization of the position of the material points: initial position  $\times$  and optimized position  $\square$ .

It appears that the optimization step is necessary as the optimal position highly depends on the initial node-set. However, for some initial position, the Newton-Raphson algorithm gets out of the domain. In such case, it has arbitrary been decided to let the material point at its original position and calculate the value of  $\lambda^*$ .

Then, a convergence study was made in order to compare the results for this optimized configuration and a calculation made with only one single material point at the center of each simplex. The results are shown on figure 3.9.

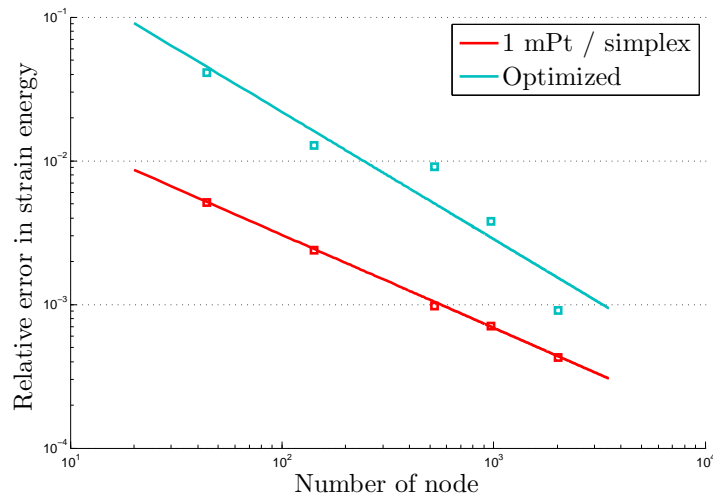
There is a significant difference of the convergence order: 0.88 for the optimized configuration against 0.65 for the classical one. However, the precision is better for a reasonable number of node. Since a Newton-Raphson algorithm has to be used in

Number of node $n$	Strain Energy $S(n)$ (J)	Relative Error
44	2.82001e9	4.0986e-2
142	2.90296e9	1.2777e-2
525	2.91371e9	9.1214e-3
973	2.92942e9	3.7788e-3
2017	2.93785e9	9.1196e-4

**Table 3.2:** Strain energy for different initial node-sets with optimization. Sref = 2.94053e9 J.

Number of node $n$	Strain Energy $S(n)$ (J)	Relative Error
44	2.98334e9	5.1034e-3
142	2.97704e9	2.3891e-3
525	2.9744e9	9.7987e-4
973	2.97411e9	7.0407e-4
2017	2.97372e9	4.28276e-4

**Table 3.3:** Strain energy for different initial node-set: one material point per simplex. Sref = 2.97317e9 J.



**Figure 3.9:** Comparison of convergence order with two different material point configurations  $\gamma = 7.2$ .

both cases, the CPU time is basically the same. Therefore, this *optimized* method will not be used further.

## 5.2 Test of different quadrature rules

Two usual quadrature rules are compared: Gauss-Hermite and Gauss-Legendre quadrature rules. In this section, 1D problem with an irregular node-set of three nodes is considered:



Let  $x_0 = 0$ ,  $x_2 = 1.3$  and  $x_1 = 2$ .  $D$  refers the elasticity matrix.

### 5.2.1 Gauss-Hermite quadrature rule

The Gauss-Hermite quadrature rule is tested since it is usually suggested for integrating exponential functions.

The MaxEnt shape functions can be written as following:

$$p_{\beta_a}(x) = f_{\beta}(x) \exp[P_2(x)] \quad (3.31)$$

where  $f_{\beta_a}(x) = \frac{1}{\log Z(x)}$  and  $P_2(x)$  is an order 2 polynomial of  $x$ . Therefore,

$$\forall(a, b) \in [1, N], \nabla p_{\beta_a}(x) \nabla p_{\beta_b}(x) = g_{\beta_{ab}}(x) \exp[-\beta x^2] \quad (3.32)$$

By the variable change  $X = \sqrt{\beta}x$  in the calculation of the stiffness matrix:

$$\forall(a, b) \in [1, N], K_{ab} = D \int_{x_0}^{x_1} \nabla p_{\beta_a}(x) \nabla p_{\beta_b}(x) dx = D \int_{\sqrt{\beta}x_0}^{\sqrt{\beta}x_1} \tilde{g}_{\beta_{ab}}(X) \exp[-X^2] dX \quad (3.33)$$

The shape functions can also be extended such as :

$$\forall a \in [1, N], p_{\beta_a}(\mathbf{x}) = \begin{cases} f_{\beta}(x) \exp[P_2(x)] & \text{if } x \in [x_0, x_1] \\ 0 & \text{otherwise} \end{cases} \quad (3.34)$$

Then:

$$\forall(a, b) \in [1, N], K_{ab} = D \int_{-\infty}^{\infty} \tilde{g}'_{\beta_{ab}}(X) \exp[-X^2] dX = D \sum_{i=1}^n w_i \tilde{g}'_{\beta_{ab}}(X_i) \quad (3.35)$$

according to the Gauss-Hermite quadrature rule and where  $n$  is the total number of material points used. The  $X_i$  are the roots of the physicists' version of the Hermite polynomial  $H_n(x)$  ( $i = 1, 2, \dots, n$ ), and the associated weights  $w_i$  are given by:

$$w_i = \frac{2^{n-1} n! \sqrt{\pi}}{n^2 [H_{n-1}(y_i)]^2} \quad (3.36)$$

Since the original function is only defined over  $[x_0, x_1]$ , a bijective application must be used to work in the proper domain. The variable change  $y = \frac{x_1-x_0}{2} \tanh(X) + \frac{x_1+x_0}{2}$  is arbitrary chosen.

$$\forall (a, b) \in [1, N], K_{ab} = D \sum_{i=1}^n J_i w_i \tilde{g}'_{\beta_{ab}}(y_i) \quad (3.37)$$

where  $J_i = \frac{x_2-x_0}{2}(1 - \tanh^2(y_i))$  if  $x \in [x_0, x_2]$  and  $J_i = \frac{x_1-x_2}{2}(1 - \tanh^2(y_i))$  if  $x \in [x_2, x_1]$  is the corresponding Jacobian.

### 5.2.2 Gauss-Legendre quadrature rule

According to the Gauss-Legendre quadrature rule:

$$\forall (a, b) \in [1, N], K_{ab} = D \int_{x_0}^{x_1} \nabla p_{\beta_a}(x) \nabla p_{\beta_b}(x) dx = D \sum_{i=1}^n J_i w_i \nabla p_{\beta_a}(x_i) \nabla p_{\beta_b}(x_i) \quad (3.38)$$

where  $n$  is the total number of material points used. The  $x_i$  are the roots of the Legendre polynomial  $P_n(x)$  ( $i = 1, 2, \dots, n$ ), and the associated weights  $w_i$  are given by:

$$w_i = \frac{2}{(1 - x_i^2) [P'_n(x_i)]^2} \quad (3.39)$$

and  $J_i = \frac{x_2-x_0}{2}$  if  $x \in [x_0, x_2]$  and  $J_i = \frac{x_1-x_2}{2}$  if  $x \in [x_2, x_1]$  is the corresponding Jacobian.

### 5.2.3 By subdivision

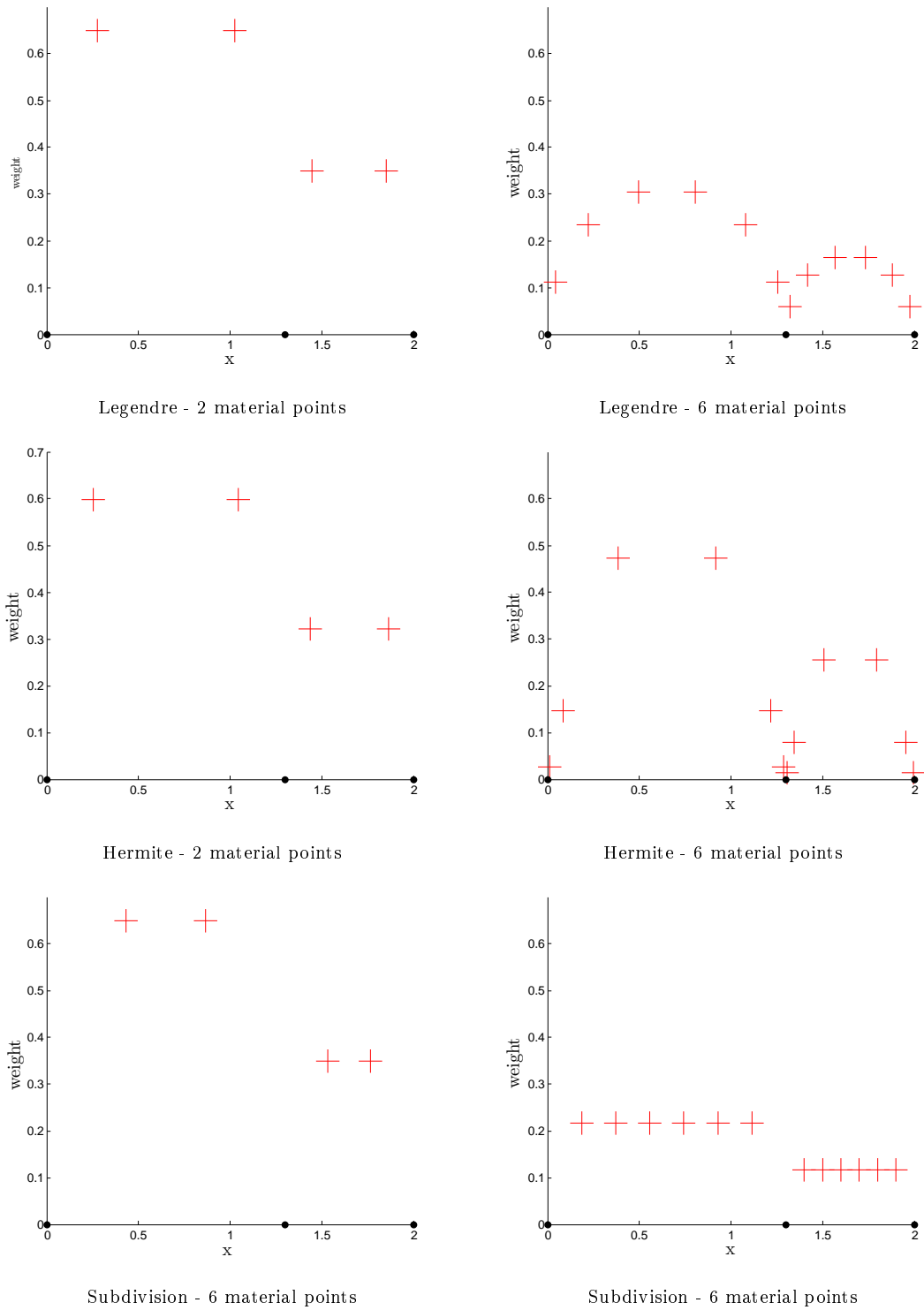
In order to compare the previous results with a *naive* way to put the material points, a distribution of regularly spaced material points over each simplex is tested.

$$\forall (a, b) \in [1, N], K_{ab} = D \int_{x_0}^{x_1} \nabla p_{\beta_a}(x) \nabla p_{\beta_b}(x) dx = D \sum_{i=1}^n J_i w_i \nabla p_{\beta_a}(x_i) \nabla p_{\beta_b}(x_i) \quad (3.40)$$

where  $n$  is the total number of material point used. The  $x_i$  are regularly spaced, and the associated weights  $w_i$  are given by:

$$w_i = \frac{2}{n} \quad (3.41)$$

and  $J_i = \frac{x_2-x_0}{2}$  if  $x \in [x_0, x_2]$  and  $J_i = \frac{x_1-x_2}{2}$  if  $x \in [x_2, x_1]$  is the corresponding Jacobian.



**Figure 3.10:** Material points position, their respective weights for different quadrature rules.

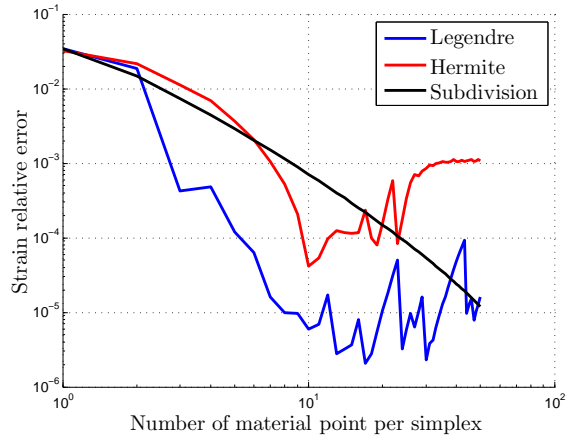
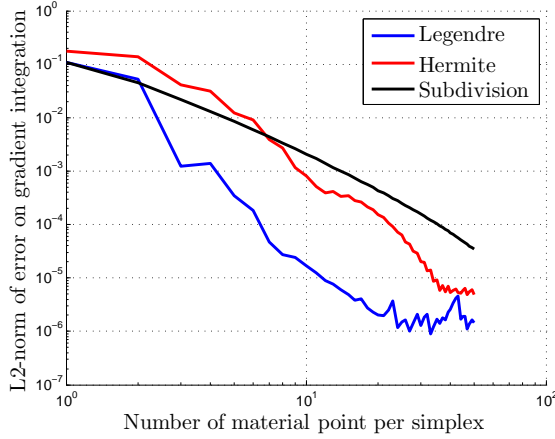
### 5.2.4 Comparison of those three quadrature rules

Figure 3.10 shows the position of the material points for each quadrature for a certain number of material points per simplex.

A first verification to check the accuracy of a quadrature is to verify that  $\forall a, \int_{x_0}^{x_1} \nabla p_{\beta_a} = p_{\beta_a}(x_1) - p_{\beta_a}(x_0)$ . Figure 3.11 represents the  $L_2$ -norm of the computed error.

Now, a deformation of 100% is imposed on the extreme nodes and the relative error in the  $L_2$  norm on the computed strain at the central node is computed: a strain of 100% at node  $x_2$  is expected. Figure 3.12 shows the strain relative error.

Those results show that the Gauss-Legendre quadrature is the most effective, even with a low number of material points. Therefore, the material points are put at the Gauss point location with the associated weight. In 2D, three material points per simplex are used.



**Figure 3.11:** Gradient integration error.

**Figure 3.12:** Strain relative error.

## 5.3 Conclusions on the quadrature rules

In this section, we looked for a possible optimized position for the material points since they can theoretically be put everywhere. A first idea was to put them where  $\lambda^* = \mathbf{0}$ . As the interpolation functions are only computed at the material points location, knowing that  $\lambda^* = \mathbf{0}$  would have allowed us to not calculate it, which could have represent a significant amount of time. However, it appeared that these particular positions defined by equation 3.23 are not easy to find since it heavily depend of the considered node set.

As shown on figure 3.8, when the node set is regular, the elements of  $M$  seem to be the middle of the grid edges and the center of the simplexes. But it is no longer the case as soon as we are looking at the borders of the convex domain  $convX$  where

the *optimized* position is not exactly on the external edge but a little inside the considered simplex. Finally, an irregular node set seem to present some difficulties since the *optimized* position are no longer on the edges.

Because of this, if we want to find the *optimized* coordinates, a Newton-Raphson algorithm has to be used and therefore, we no longer gain the time we would have earned by not computing  $\lambda^*$ . Moreover, figure 3.9 shows that the *optimization* does not give better results in term of convergence and accuracy.

Then, the classical quadrature rules have been compared in order to find out which one is the more suitable for our implementation. It turns out that the very classical Gauss-Legendre quadrature, which is used in FEM to obtain the Gauss points, gives the better results for a low number of material points per simplex.

For all these reasons, the Gauss-Legendre quadrature has been selected to put the material points in our implementation.

## 6 Conclusion

In this chapter, the Local Maximum Entropy (LME) approach has been presented. This approach allows to build a meshless method by using material points (see chapter 1). This approach comes with very interesting features such as the possibility to put precise boundary condition (contrary to the most common meshless methods, see chapter 1) and therefore to manage contact. Theoretically, it should also be possible to handle auto-contact simply by updating the neighborhoods as explained in Appendix A.

The main point of interest of this chapter is the discussion about the position of the material points. The conclusion of the study is that it does not seem to be an easy way to choose the coordinates of the material points *smartly* by saving computational time. Also, a comparison between standard quadrature rules has been done and the Gauss-Legendre quadrature has been selected as the rule to put the material points in our implementation.

Finally, this implementation seems to be highly dependent on the choice of the parameter  $h$  (and consequently on  $\gamma$ ) since it has a high impact on the shape functions (see figure 3.2). This dependence will be studied in the next chapter.

# Chapter 4

## Validation

*In this chapter, the implementation of the MaxEnt interpolation is verified through some test cases. First, we test it with a simple conduction problem. Then a patch test used in the literature will allow us to discuss about the convergence depending on the node set and the number of material points. Finally, the classic Taylor bar case will allow us to discuss about the numerical locking phenomena depending on the  $\beta$  parameter, the initial node set and the number of material points.*

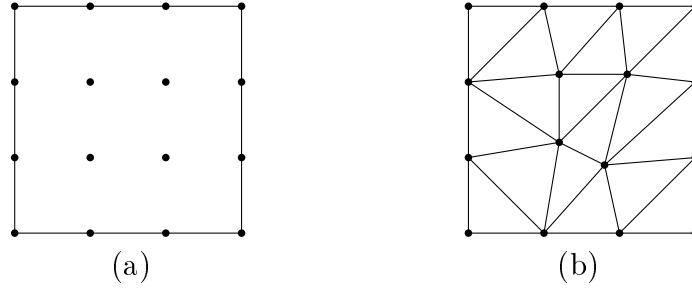


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**Figure 4.1:** Patch-test cases.

	Regular node-set (a)		Irregular node-set (b)			
mPt	$\gamma = 9.44$	$\gamma = 1.8$	$\gamma = 9.44$	$\gamma = 8.02$	$\gamma = 1.8$	$\gamma = 0.8$
1	6.50e-10	1.29e-5	7.51e-2	7.22e-2	4.16e-2	5.30e-2
3	4.18e-11	2.86e-4	2.37e-3	1.54e-3	3.01e-2	5.88e-2

**Table 4.1:** Relative errors in the  $L_2$ -norm for the deformation gradient at mPts.

## 1 Patch test

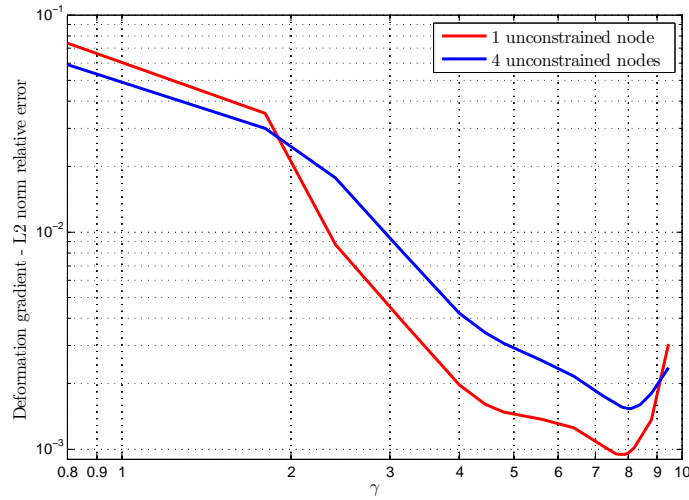
Our implementation of the MaxEnt interpolation is submitted to the same patch test as in [Arroyo and Ortiz, 2006]. In the displacement patch test, the boundary of the computational domain is subjected to an affine transformation. For the numerical method to pass the test, the numerical solution in the interior of the domain must reproduce this affine transformation exactly. Consider the square  $[0, 1] \times [0, 1]$  of a linear isotropic elastic material characterized by Young modulus  $E = 1Pa$  and the Poisson's ratio  $\nu = 0.3$ . The boundary of the square is subjected to a linear transformation characterized by the matrix

$$\begin{pmatrix} 1 & -\sqrt{3}/2 \\ \sqrt{3} & 1/2 \end{pmatrix}$$

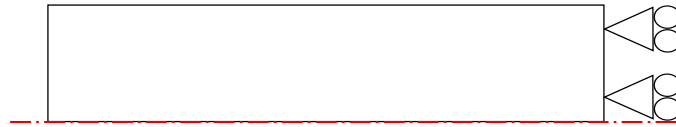
The two considered node-sets are described in 4.1. In both cases, simplices are obtained by using a Delaunay triangulation on the node-set. The material points are given by the Gauss-Legendre quadrature and there are three material points per simplex.

The  $L_2$ -norm of the error on the deformation gradient at material points is computed. Table 4.1 shows the obtained results for cases *a* and *b* for different values of  $\gamma$ .

It appears that the precision is depending on the initial node-set but also on the  $\gamma$  parameter. The figure 4.2 shows the  $L_2$ -norm relative error as a function of  $\gamma$  but also of the boundary conditions. The case with the four unconstrained nodes is the previously described case (b). For one unconstrained node, the case (b) of an



**Figure 4.2:** L2-norm error for the patchtest depending on  $\gamma$  and the boundary conditions.

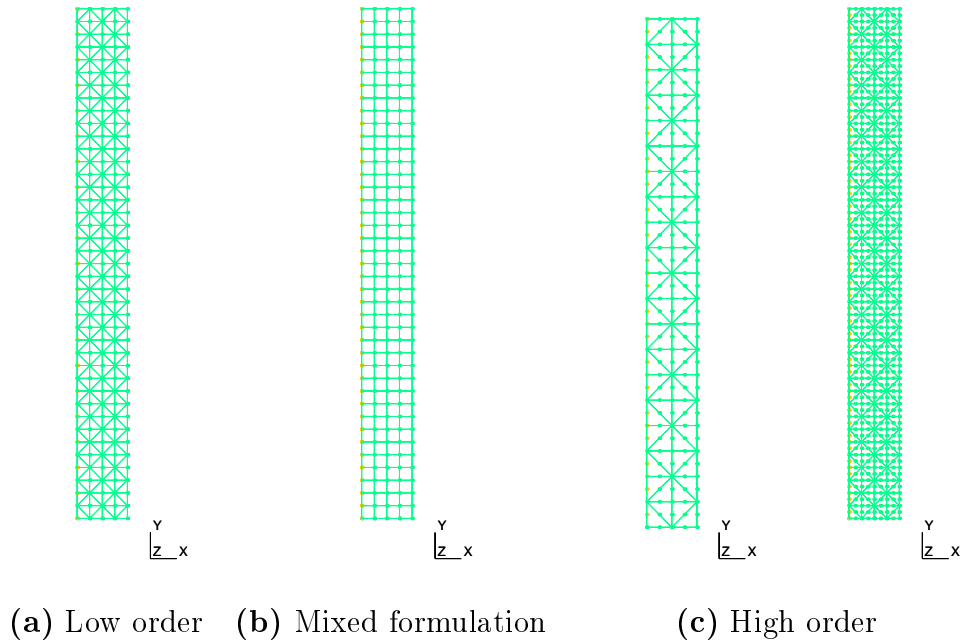


**Figure 4.3:** Taylor bar - boundary conditions

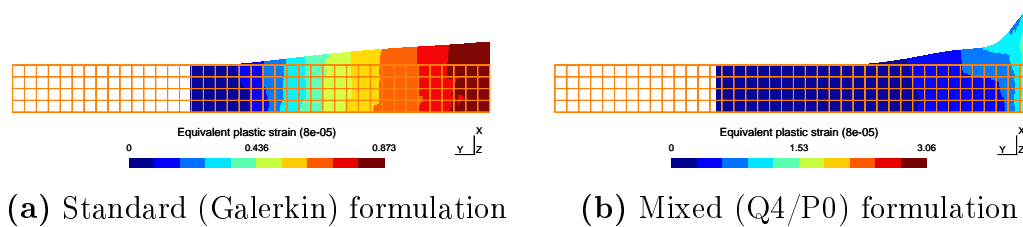
irregular node-set is also considered but the transformation is applied to every node except the upper right inner node. It appears that the minimum in terms of relative error on the deformation gradient is depending on  $\gamma$  but also on the boundary conditions. In this last case, the minimum is obtained for  $\gamma = 7.71$  whereas it is obtained for  $\gamma = 8.02$  in the four unconstrained nodes case. Even if the problem is not exactly the same,  $\gamma$  is only a function of  $h$ , which only depend on the node set so this different may be a little surprising, even if the difference is not so big. The conclusion of this patchtest analysis is that in our implementation, the optimal  $\gamma$  is very dependent on the node set.

## 2 Taylor bar

This section will present the classical Taylor bar compressive impact test. We consider an axi-symmetric cylinder of copper with a high initial velocity in the  $y$ -direction. The material model is linear isotropic elastic and large deformations are considered. Nodes such as  $y=0$  are fixed in  $y$ -direction. A classical discussion about this test is the presence of the plastic volumetric locking.



**Figure 4.4:** Options to avoid locking with standard FEM

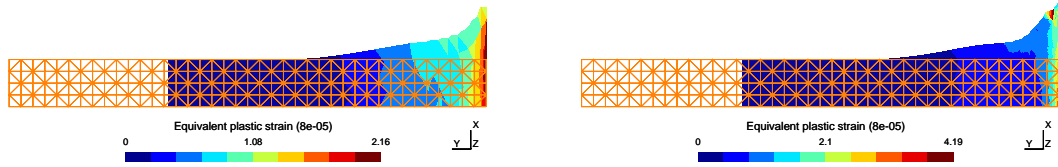


**Figure 4.5:** Locking on quad elements

## 2.1 Locking with FEM

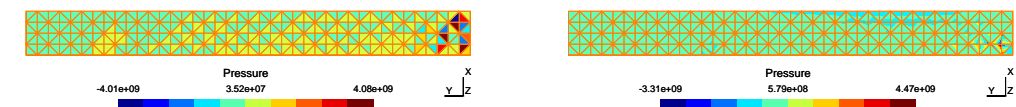
The impact test is done with three different meshes shown on figure 4.4. We use low order elements such as T3 or Q4, a mixed formulation and finally the use of higher order elements, typically quadratic triangles with 6 nodes (T6) with more nodes.

For quadrangles, a mixed formulation Q4/P0 is necessary to avoid locking as shown on figure 4.5. On the contrary, the figure 4.6 use of simplicial elements seems to reduce the impact of the locking phenomena. In every case, the locking leads to checkerboard pressure modes as shown on figure 4.7.

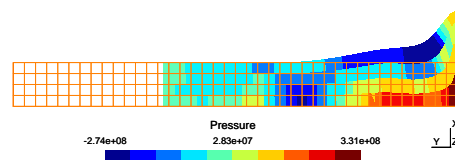


(a) Linear elements seem to work here (b) Second order triangles (more nodes)

**Figure 4.6:** Locking on simplicial elements

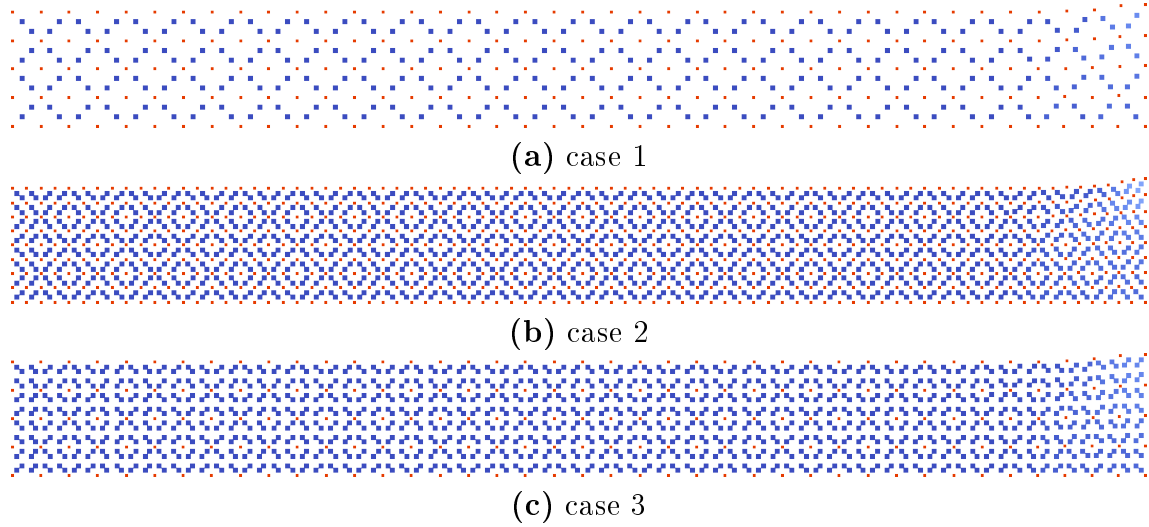


(a) T3 (undeformed configuration) (b) T6 (undeformed configuration)



(c) Q4/P0

**Figure 4.7:** Locking on simplicial elements



**Figure 4.8:** Locking with MaxEnt: three studied cases

## 2.2 Locking with MaxEnt

Now, the MaxEnt interpolation is used. We choose to use a total Lagrangian approach. The interpolation functions are computed once and for all in the reference configuration, they are not updated. Finally, as explained in the previous chapter, the integration is performed at material points. The objective is to find out the influence on the locking phenomena of the thermalization parameter  $\gamma$  and the density of material points, especially with respect to volumetric locking.

In the first case, we consider a simple initial node set with a density of material points of one per simplex. In the second case, we use a more dense initial node set and keep one single material point per simplex. In the last case, we use the first node set but have four material points per simplex. The three cases are shown on figure 4.8 where the blue squares are the material points and the small red one are the nodes. With those three cases, we successively set the  $\gamma$  parameter equal to 0.5, 1.0 and 1.8.

It appears that the case 3 (high density of node and four material points per simplex) is the most likely to provoke locking. As soon as  $\gamma \geq 1.0$ , the locking is very visible. On the other hand, it seems that with only one material point per simplex, the density of node do not have an influence on the apparition of locking.

Concerning the pressure field, it appears that the case 3 leads to a *checkerboard* pressure field, which is shown on figure 4.11-f. The two other cases have a regular pressure field and it is similar to each other. In the three cases, the pressure fields are very similar to the one obtained by using finite element method.

We compare in table 4.2 the final length of the rod, the final mushroom radius, the

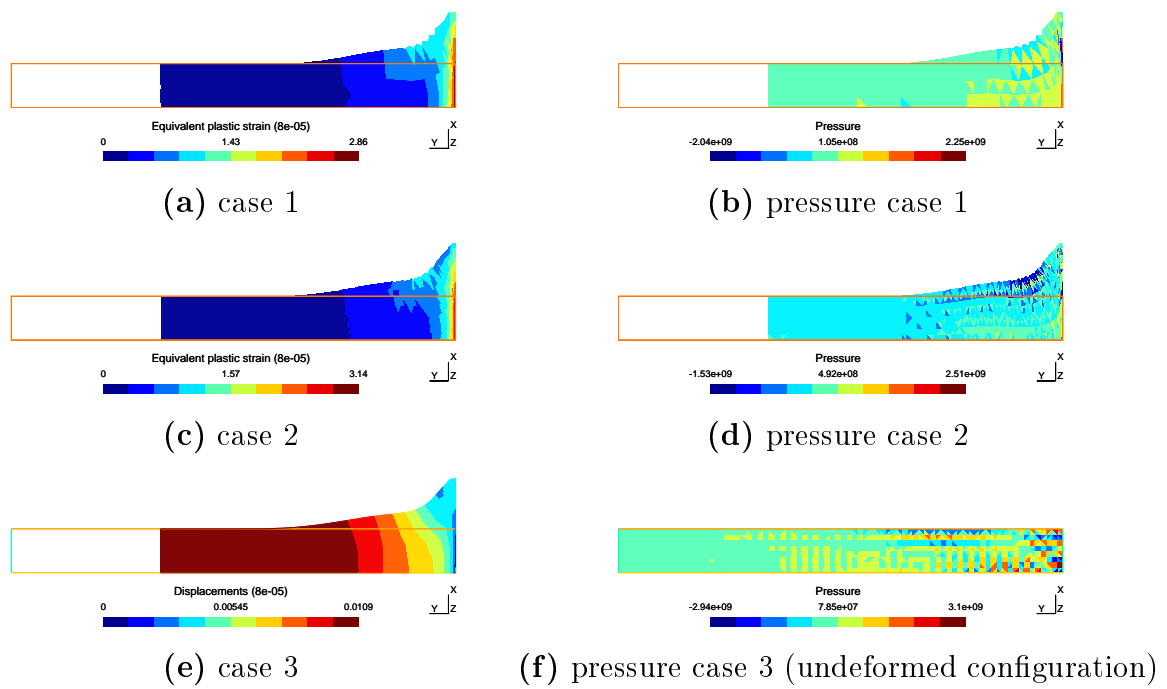


Figure 4.9: Locking with MaxEnt:  $\gamma = 0.5$

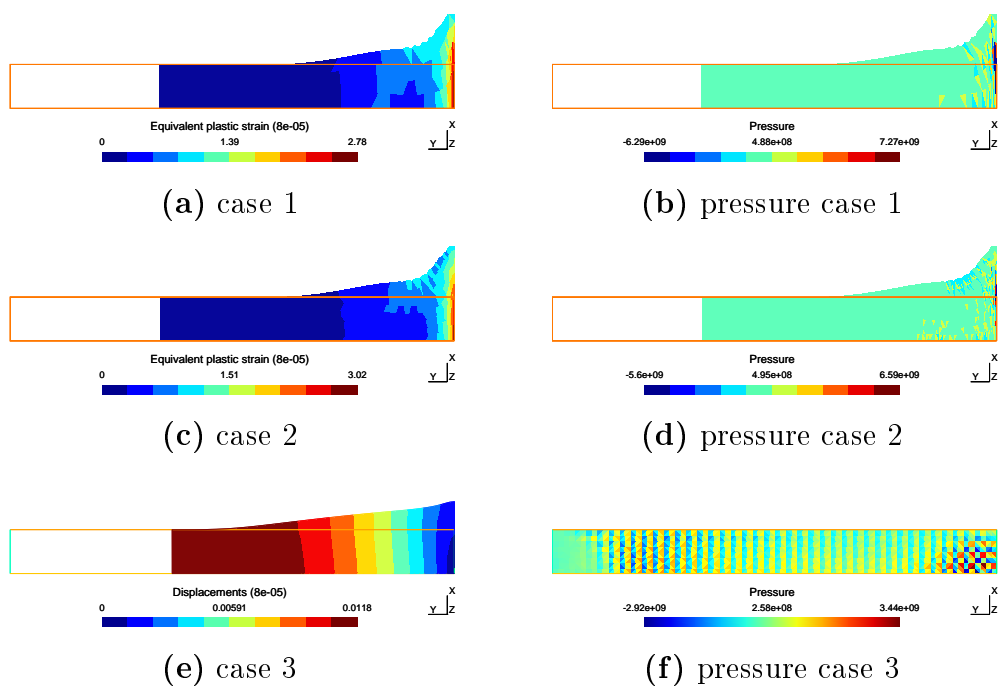
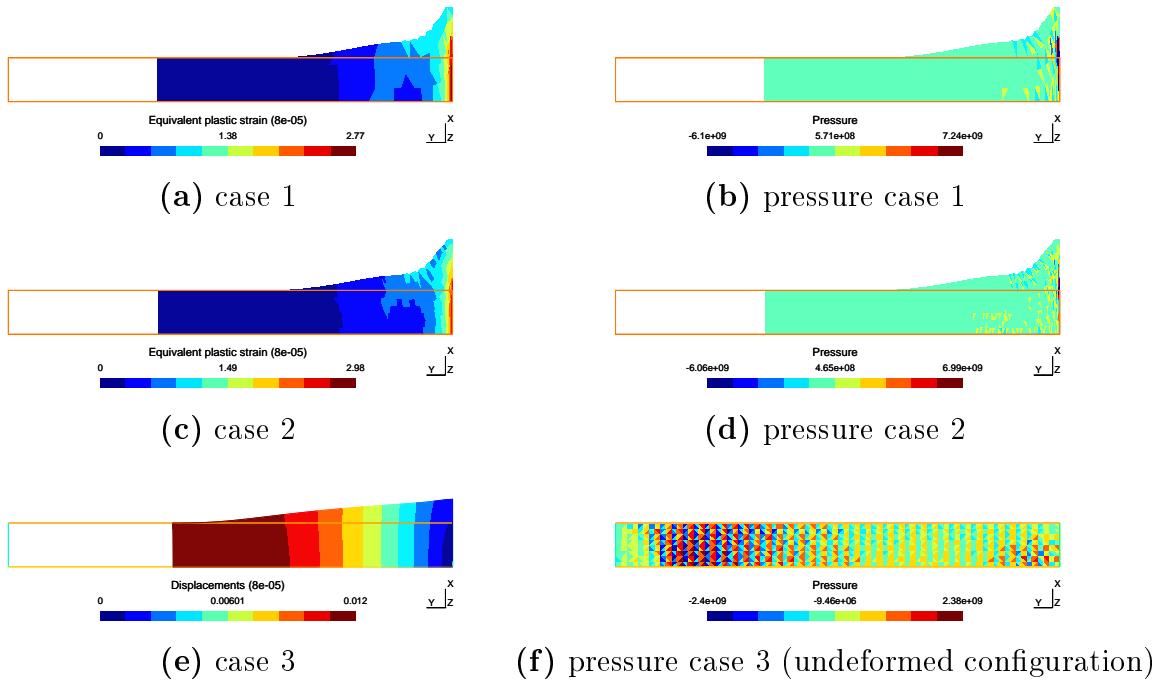


Figure 4.10: Locking with MaxEnt:  $\gamma = 1.0$ , appearance of locking



**Figure 4.11:** Locking with MaxEnt:  $\gamma = 1.8$ , appearance of locking

maximum effective strain and von Mises stress obtained by different methods. All the results are obtained in an axi-symmetric context. Our implementation obtains excellent agreements with the other results.

The conclusion is that depending on the  $\gamma$  parameter, the node density and the number of material points, it is possible to avoid the locking phenomena, which is a very important feature for the modeling of many manufacturing processes, in particular the RFW process. Moreover, the results are reliable and agree with results obtained with other methods.

### 3 Unilateral contact

A penalty method has been implemented to manage the contact between a deformable body modeled using a LME formulation and a rigid body. In this classical method, the gap between the considered node and the rigid body is computed:

$$g = (\mathbf{x}_a - \mathbf{x}_{rigid}) \cdot \mathbf{n} \quad (4.1)$$

where  $\mathbf{n}$  is normal to the rigid surface.

The penalty algorithm is the following. At current time step for each node, the gap  $g$  is computed. If  $g > 0$ , there is no contact. On the contrary, if  $g < 0$ , there is a



	Final length (mm)	Final mushroom radius (mm)	Max. effective plastic strain	Max von Mises stress (MPa)
[Kamoulakos, 1990]	21.47 – 21.66	7.02 – 7.12	2.47 – 3.24	472 – 476
[Zhu and Cescotto, 1995]	21.26 – 21.49	6.89 – 7.18	2.75 – 3.03	419 – 477
[Camacho and Ortiz, 1997]	21.42 – 21.44	7.21 – 7.24	2.97 – 3.25	/
OTM [Li, 2009]	21.43	6.8	3.0	474
LME	21.50	6.81	2.69	516

**Table 4.2:** Taylor anvil impact test: comparison of results.

small penetration and a penalty force has to be applied. The first thing done is the computation of the penalty force:

$$F_p = C.E.g \quad (4.2)$$

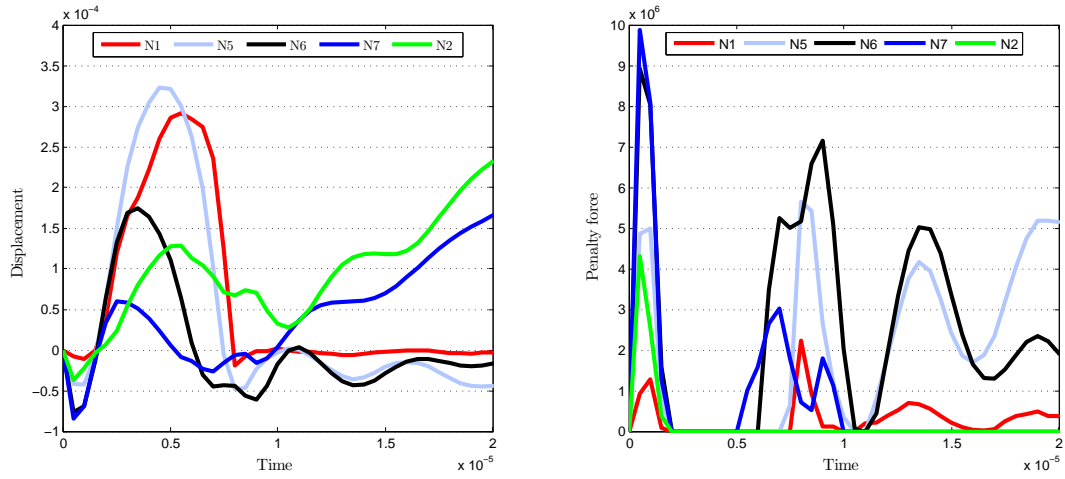
where  $C$  is the penalty coefficient and  $E$  the Young modulus of the material. This force is applied to the considered node and the penalty coefficient times  $E$  is added to the tangent matrix. The influence of the penalty coefficient is studied.

In this study, the geometry, material and initial boundary conditions are the same as in the study of the Taylor's bar and we use the case 1 configuration (one material point per simplex and small density of node) with  $\gamma = 1.8$ . The boundary conditions are presented on figure 4.3. The considered material is copper and the initial high velocity is  $-227 \text{ m.s}^{-1}$  on the  $y$ -direction. We consider a contact between the bar and a rigid body surface defined by  $y = 0$ . Therefore, an unilateral condition has been added for the five nodes of coordinate susceptible to penetrate the rigid body. Finally, the simulations are run in implicit (statics or dynamics).

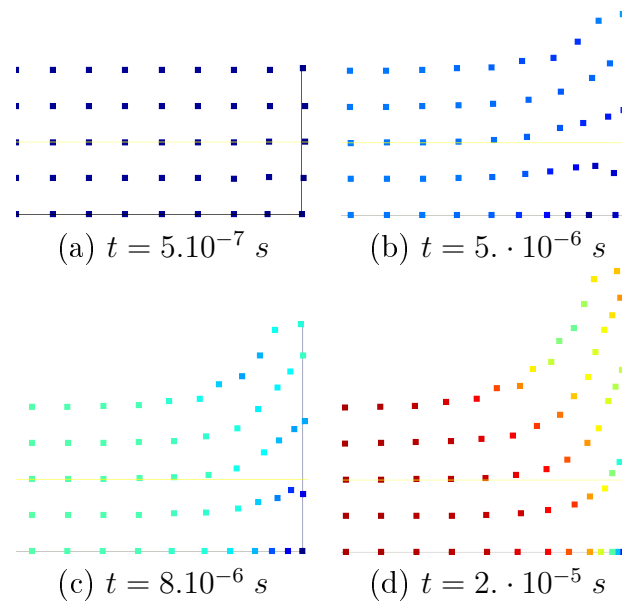
In a first case, we chose  $C = 1 \text{ m}$ . Figures 4.12 and 4.13 represent the evolution of the gap during the first  $20 \mu\text{s}$  for the five nodes at the contact interface. Their initial  $y$ -coordinates are given in the following table:

$y_{N1}$	$y_{N5}$	$y_{N6}$	$y_{N7}$	$y_{N2}$
0.	0.0081	0.0162	0.0243	0.0324

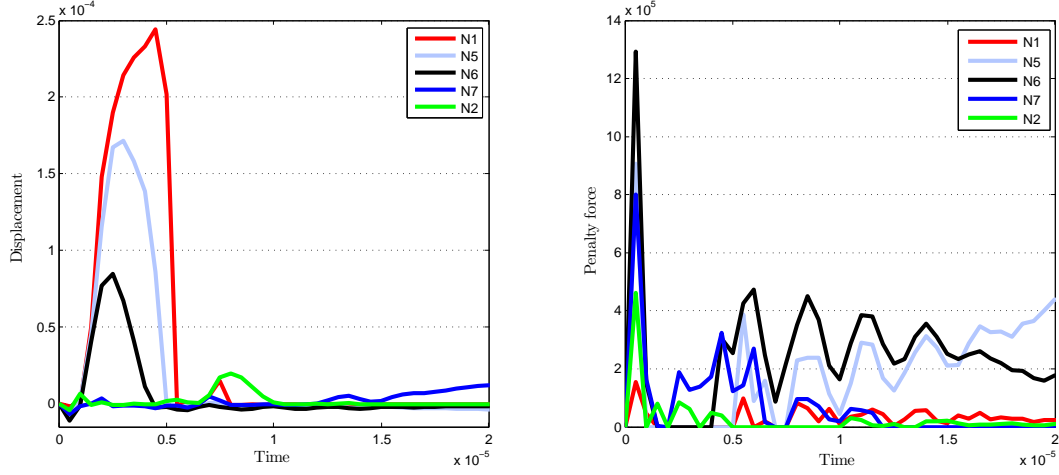
The figure 4.12 reveals three distinct time intervals in the evolution of the gap. At the first time step, there is a penetration  $g < 0$ . The penalty force is applied to prevent this. Due to this force, the gap becomes positive  $g > 0$  and there is a *rebound*. After this rebound, a small penetration remains (around  $2.5 \cdot 10^{-5} \text{ m}$ )



**Figure 4.12:** Evolution of the gap function and penalty force for a penalty coefficient  $C = 1 \text{ m}$  ( $CE = 117 \cdot 10^9 \text{ N.m}^{-1}$ ).



**Figure 4.13:** Node evolution with a penalty coefficient  $C = 1 \text{ m}$ .



**Figure 4.14:** Evolution of the gap function and penalty force for a penalty coefficient  $C = 10 \text{ m}$  ( $CE = 117. \cdot 10^{10} \text{ N.m}^{-1}$ ).

which tends to prove that the penalty force may not be large enough.

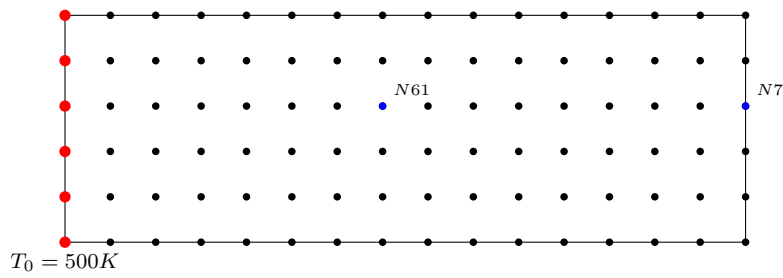
The figure 4.14 shows the results of this same simulation with a penalty coefficient  $C = 10 \text{ m}$ . The same three time intervals are observable. At first there is a penetration  $g < 0$  that causes a penalty force. Again, it leads to a positive gap  $g > 0$  and there is a *rebound*. The first remark is that the *rebound* characteristics have changed:

- the size of the *rebound* is smaller than previously:  $g_{C=10}^{max} \simeq 1.5 \cdot 10^{-4} \text{ m}$  whereas  $g_{C=1}^{max} \simeq 3.25 \cdot 10^{-4} \text{ m}$
- the penalty force is smaller than previously:  $F_{C=10}^{max} \simeq 1.3 \cdot 10^6 \text{ N}$  whereas  $F_{C=1}^{max} \simeq 1. \cdot 10^7 \text{ N}$ .
- the duration of the *rebound* is smaller than previously:  $t_{r,C=10} \simeq 0.6 \cdot 10^{-5} \text{ s}$  whereas  $t_{r,C=1} \simeq 0.8 \cdot 10^{-5} \text{ s}$ .

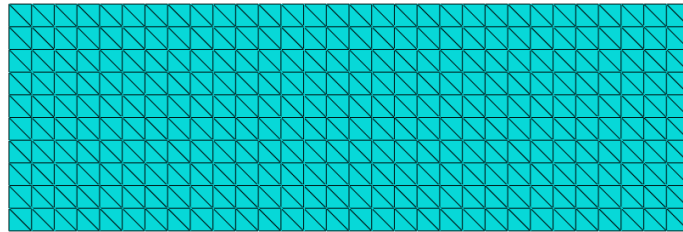
Concerning the period of time after the end of the rebound, we can see on the penalty force graph that a small penetration still remains (around  $1.4 \cdot 10^{-7} \text{ m}$ ). This lingering penetration is small enough to be considered as acceptable.

## 4 Heat conduction

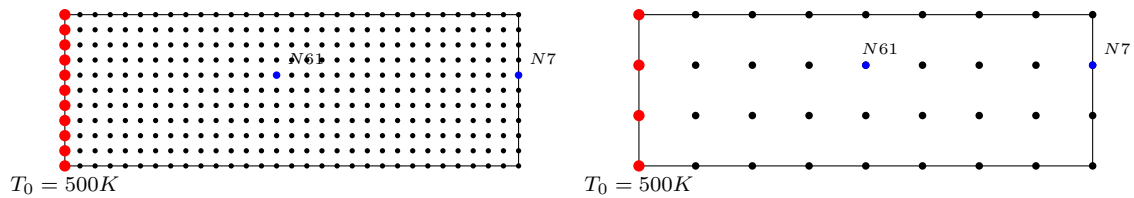
The first test case implying temperature is a pure conduction problem. Consider a plate of  $30\text{cm}$  over  $10\text{cm}$ . The considered material has the following properties: density  $\rho = 7850 \text{ kg.m}^{-3}$ , conduction coefficient  $\lambda = 46 \text{ W.m}^{-1}.\text{K}^{-1}$  and specific



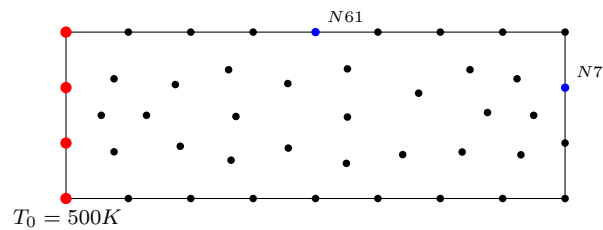
**Figure 4.15:** Initial conditions for conduction problem.



**Figure 4.16:** Mesh used for Abaqus FEM simulation of the conduction test.



**Figure 4.17:** Refined mesh for conduction problem. **Figure 4.18:** Coarse mesh for conduction problem.



**Figure 4.19:** Irregular mesh for conduction problem.

heat capacity  $c_p = 500 \text{ J.K}^{-1}$ . The initial node-set and related  $convX$  are shown on figure 4.15.

The initial and boundary conditions are:

$$T(x, y, 0) = 0 \quad (4.3a)$$

$$\dot{T}(x, y, 0) = 0 \quad (4.3b)$$

$$T(0, y, t) = T_0 \quad (4.3c)$$

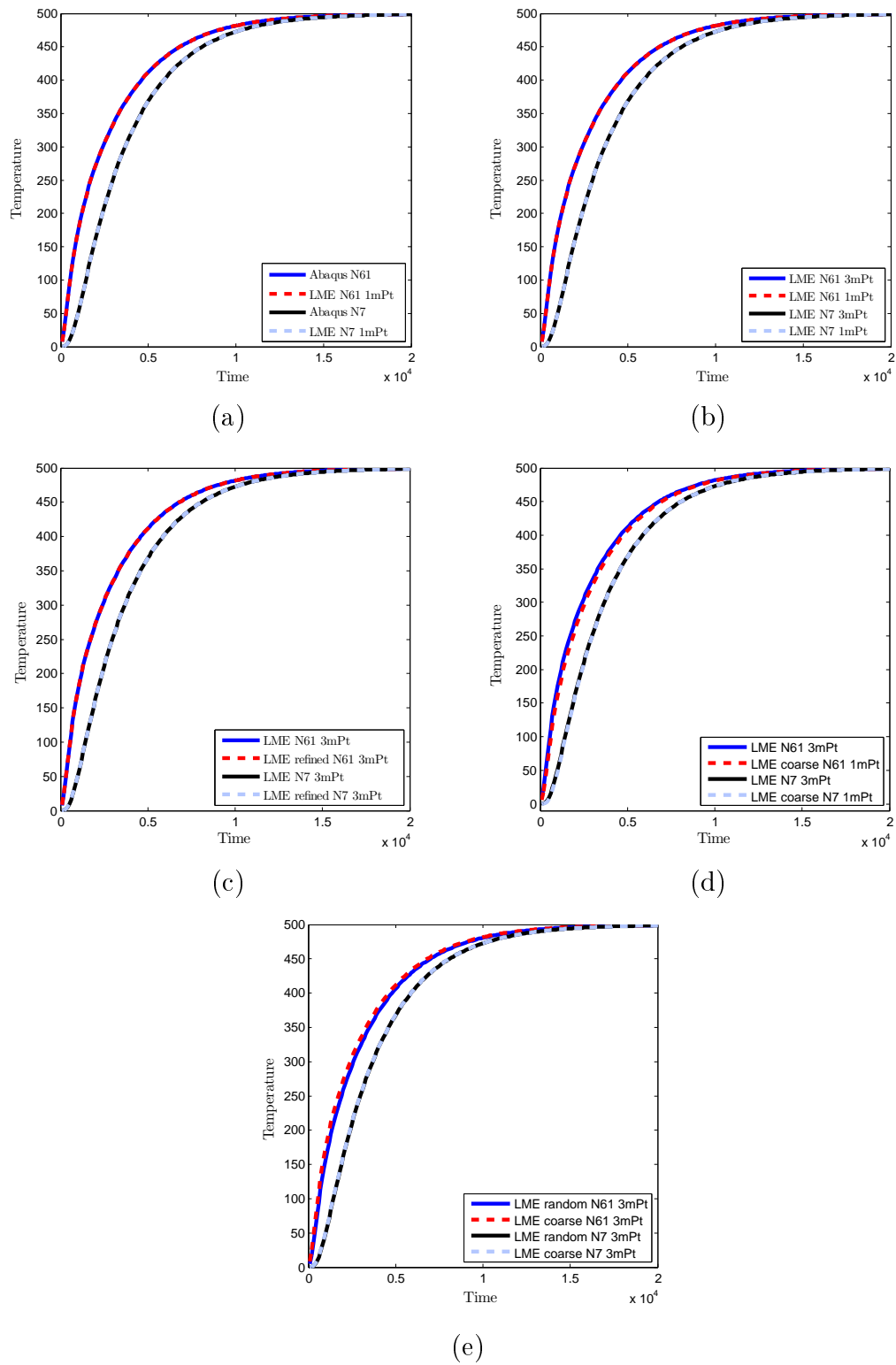
The objective here is to compare the temperature evolution obtained using the LME interpolation with the results obtained with Abaqus using a FE approach. The influence of the density of node and their position will also be studied.

The temperature evolution over time is displayed at the positions of nodes 61 and 7 on the initial configuration as shown on figure 4.15. For the other configurations, the temperature will be measured at this same location. The analytic solution is only known for a semi-infinite system, therefore the results cannot be compared to any analytic solution. In order to have a reliable solution, the mesh for the FE computation is chosen twice as thin.

Figure 4.20 shows the temperature evolution at the two chosen positions for a variation of node set and number of material points:

- a : Comparison of conduction results obtained using LME and FEM (figures 4.15 and 4.16).
- b : Comparison of conduction results for different number of material points per simplex (figure 4.15).
- c : Comparison of conduction results for the initial mesh and a refined mesh (figures 4.15 and 4.17).
- d : Comparison of conduction results for the initial mesh and a coarser mesh (figures 4.15 and 4.18).
- e : Comparison of the conduction test a regular and an irregular (figures 4.18 and 4.19).

It appears for a node set dense enough, the results are exactly the one obtained using Abaqus, and this independently of the number of material point. On the other hand, if the node set is not dense enough, we can see a small difference in the temperature evolution at position N61. This difference is considered small enough to be acceptable. Therefore, LME and our implementation succeeded this test.



**Figure 4.20:** Comparison of the conduction test for different configurations

## 5 Convection

A coupled thermo-mechanical problem is now considered. In this test, a thermo-plastic model including large deformations and non-linear hardening is used. The material is a mild-steel with the properties described in table 4.3. The boundary conditions are shown on picture 4.21. The problem is axi-symmetric. A displacement is applied on each side of the mild-steel cylinder. The initial temperature of the cylinder is  $T_0 = 20^\circ C$  and the outside gas (air) is at a constant temperature also equal to  $20^\circ C$ . The convective heat flux is computed as:

$$\phi_{conv} = C_i(T_{surf} - T_{air}) \quad (4.4)$$

where:

- $C_{i=1} = 15 \text{ W.m}^{-2}.K^{-1}$  on the free side.
- $C_{i=2} = 52 \text{ W.m}^{-2}.K^{-1}$  on the pulled sides.

The thermal loading applied to a node  $x_a$  submitted to a convection boundary condition is:

$$\psi_{conv,a} = C_i \frac{T_{surf} - T_{air}}{T_0} S_a dt \quad (4.5)$$

where  $S_a$  is the equivalent surface represented by node  $x_a$  which is described in chapter 5 on figure 5.3.

The simulation is during 50s. The initial state is given by figure 4.22 for two different refining level of the node-set. During this phase, displacements  $u(t)$  and  $-u(t)$  are respectively imposed on the left and right sides of the cylinder. At  $t = 50s$ ,  $|u(t = 50s)| = |u_{max}| = 3.85 \text{ cm}$ . During this phase, there are plastic deformation inside the cylinder, which imply a rise of the inner temperature. On the other hand, there is convection with the outside air. Therefore, the edges are cooler than the center. At  $t = 50s$ , figure 4.23 is obtained.

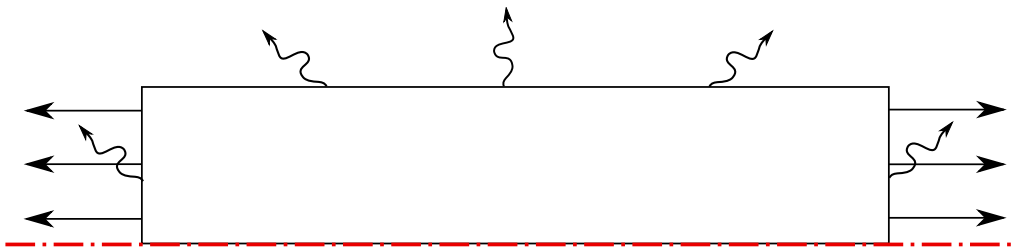
With a refined node-set, a necking appears at the center of the cylinder, just as it is expected to. The convection boundary condition on the faces of the cylinder brings a heterogeneity in the temperature field: the center of the part is hotter so the material is softer and deforms more. Since the high deformation is very localized at the center of the cylinder, a refined node-set is necessary to observe the necking.

For comparison, figure 4.24 shows the results obtained with a FE modeling using Abaqus at the end of the first step ( $t = 50 \text{ s}$ ).

However, it should be noticed that the results given by Abaqus may not be reliable for this test case. Indeed, the dependence of the elastic deformation on

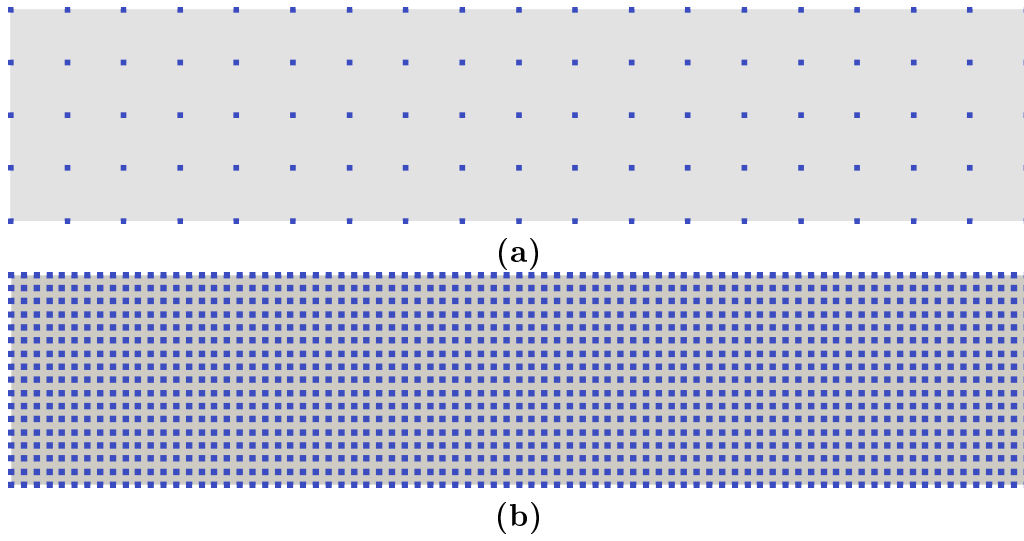
Parameter	Value
Young modulus	200 <i>GPa</i>
Poisson coefficient	0.29
Plastic properties	
Initial Yield stress	360 <i>MPa</i>
Hardening coefficient	280. <i>MPa</i>
Reference stress	0 <i>MPa</i>
Reference strain rate	0.05 <i>MPa.s<sup>-1</sup></i>
Rate dependency coefficient	0.05
Thermal properties	
Conductivity coefficient	52 <i>W.m<sup>-1</sup>.K<sup>-1</sup></i>
Specific heat capacity	470 <i>J.K<sup>-1</sup></i>
Thermo-elastic properties	
Thermal dilatation coefficient	10 <sup>-5</sup> <i>K<sup>-1</sup></i>
Thermo-plastic properties	
Ratio Dissipated energy / Total plastic energy	1.

**Table 4.3:** P265GH - Johnson-Cook model

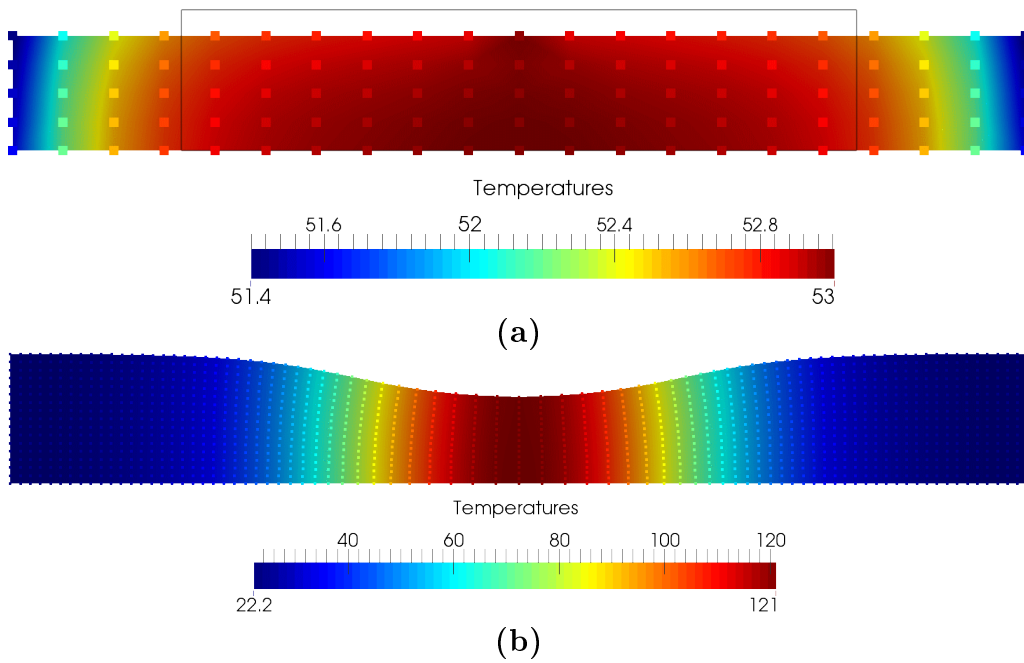


**Figure 4.21:** Convection problem - boundary conditions

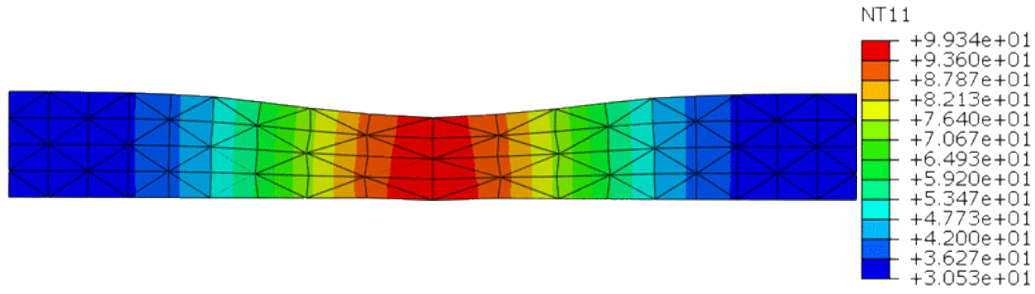




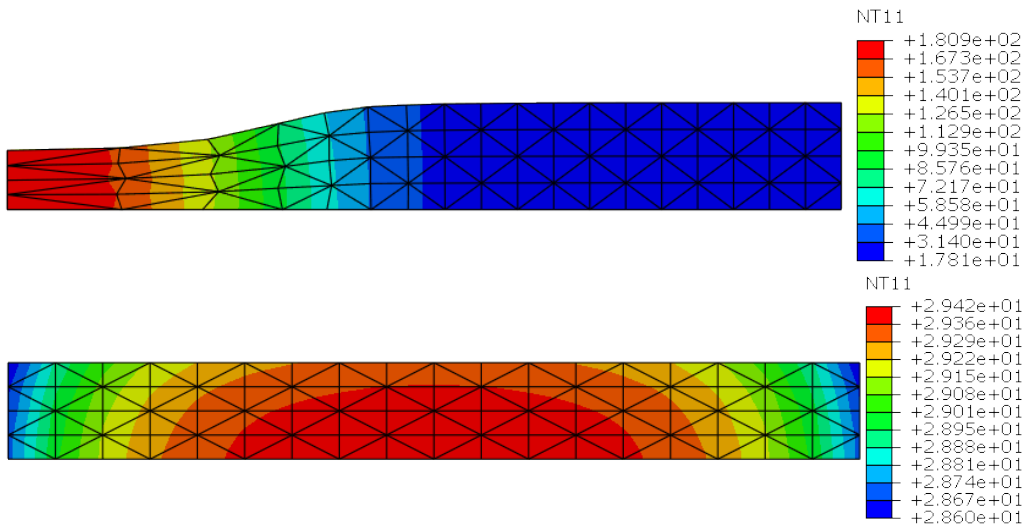
**Figure 4.22:** Convection problem - Initial state for two different node-set.



**Figure 4.23:** Convection problem - temperature - end of the traction phase with the two different node-set.



**Figure 4.24:** Convection test case using Abaqus (FE).



**Figure 4.25:** Abaqus results for a different imposed maximum time step (respectively 0.1 s and 50 s).

the temperature is not implemented in (version 6.13 was used) and it appears that imposing a small maximal time step leads to aberrant results as shown on figure 4.25 as an implicit scheme is used. The result given by figure 4.24 is obtained by changing only an imposed maximal time step of 0.5 s.

The results are reasonable compared to what is obtained with Abaqus: LME succeeded this test.

## 6 Conclusion

In this chapter, the implementation of the LME interpolation approach has been submitted to a few test cases. These tests were including some thermal, mechanical and coupled thermo-mechanical problems.

The LME interpolation approach and the variational framework have been correctly implemented since the results obtained for each test are pretty good. Yet, some dependencies of the accuracy of the solution have been observed. It appears that the implementation is particularly dependent on the regularity of the node set as shown by the patch test example. The choice of  $\gamma$  is not easy and the tuning of this parameter can highly improve the quality of the results. Moreover, tuning  $\gamma$  allows to avoid the locking phenomena, as is the density of the node set and the density of material points.

Then, we have verified the possibility to put contact boundary condition using a LME approach. At the moment, only the penalty method has been implemented and tested but a good perspective is to try to use a Lagrangian multiplier method or even an augmented Lagrangian method.

Our implementation is also able to solve purely thermal problems as shown with the conduction test case. The results are excellent and do not seem to be dependent on the node set and the material point density. The convection test case proved that it is possible to handle coupled thermo-mechanical problems with large deformation.

## Chapter 5

# Rotary Frictional Welding modeling

*The modeling of the RFW process requires a few features such as the computation of the heat flux related to the penalty force due to contact. The computation of this flux implies the existence of some other features such as the definition of an equivalent surface represented by a node. The frictional contact also has to be adjusted in order to be compatible with the variational framework, which requires a symmetric formulation. Finally, a simulation of a particular welding is presented and the results are discussed.*

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## 1 Hypothesis

A few hypotheses are made in order to model the RFW process.

The computation is not performed in three dimensions but in an axis-symmetrical context, assuming that the different parameters, such as strain, stress, temperatures, etc., do not depend on the angular position. This assumption is quite well verified in reality.

A quasi-static analysis is used, assuming that the inertia effects are negligible.

An other hypothesis is that the surface of contact is a plane defined by  $y = 0$ . Therefore the contact is modeled between a deformable part and a rigid body. This hypothesis implies that the material is homogeneous and that there is no local penetration from one welded part into the other one, which is automatically verified as long as the two parts are made of the same material. If the two parts are made of two dissimilar materials, this hypothesis may be verified if both materials are homogeneous.

The used friction law is a classical Coulomb friction law with a friction coefficient depending on the temperature. The tangent forces at the interface are neglected.

The power of friction is considered to be entirely transformed into heat. This heat is assumed to be equally divided between the two parts.

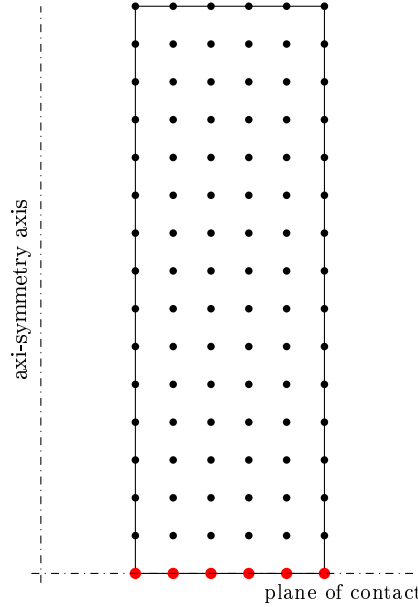
The convection between the parts and the ambient air is not taken into account.

Figure 5.1 is a scheme of the initial configuration.

## 2 Implementation of the frictional heat flux

This section deals with the most critical part in the modeling of the RFW process. The modeling of the friction heat is indeed determinant in the modeling of the process. The temperature is close to the melting point in the welded area. Since the material has very different behaviors depending on the temperature, it is important to have an as *accurate* as possible temperature field.

In this section, the considered configuration is described in its initial state on figure 5.1. This case represents a hollow cylinder. The axi-symmetric axis is defined by  $x = 0$ , a contact plane is defined by  $y = 0$ .



**Figure 5.1:** Initial configuration for the RFW modeling

## 2.1 Contact using penalty method

The classical penalty method is used to manage the contact. The gap  $g$  for a node  $x_a$  is defined:

$$g_a = \text{dist}(\mathbf{x}_a - \mathbf{P}_{\text{SYM}}) \quad (5.1)$$

where the operator  $\text{dist}(\mathbf{x}_a, \mathbf{P}_{\text{SYM}})$  computes the distance between the node  $\mathbf{x}_a$  and the plane of symmetry  $\mathbf{P}_{\text{SYM}}$ . In this case, the plane of symmetry is defined by the equation  $y = 0$ . Therefore, the gap  $g_a$  is simply defined by:

$$g_a = y_a \quad (5.2)$$

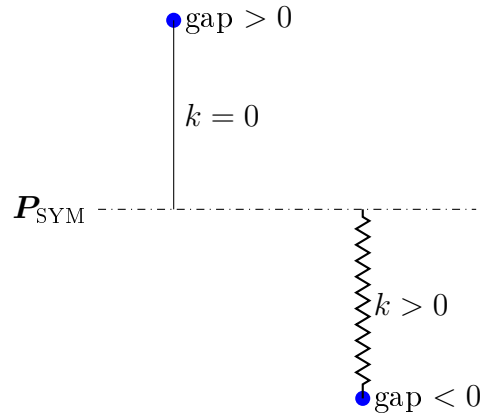
with the convention that  $g_a < 0$  means that there is a penetration as shown on figure 5.2.

According to the penalty method, a force is applied on each considered node depending on the gap:

$$\mathbf{F}_{a,P} \cdot \mathbf{n} = \begin{cases} 0 & \text{if } g_a \leq 0 \\ -kg_a & \text{if } g_a > 0 \end{cases} \quad (5.3)$$

where  $k = CE$  is the penalty coefficient and  $F$  the force applied on the considered node which is orthogonal to the plane of symmetric; here the force is over  $\vec{e}_y$ . The potential energy related to the penalty force is

$$P_{a,p} = -\frac{1}{2}ky_a^2 \quad (5.4)$$



**Figure 5.2:** Gap and penalty coefficient.

The term  $\frac{\partial^2 F_{a,p}}{\partial y^2} = -k$  must be added to the tangent matrix at the proper place (diagonal term associated to the gradient force of node a).

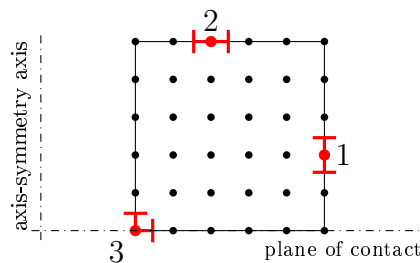
As a reminder, the tangent matrix in a variational framework must be symmetric and is defined by

$$\begin{bmatrix} \frac{\partial F_x}{\partial x} & \frac{\partial F_x}{\partial y} & \frac{\partial F_x}{\partial T} \\ \frac{\partial F_y}{\partial x} & \frac{\partial F_y}{\partial y} & \frac{\partial F_y}{\partial T} \\ \frac{\partial Q}{\partial x} & \frac{\partial Q}{\partial y} & \frac{\partial Q}{\partial T} \end{bmatrix} \quad (5.5)$$

## 2.2 Equivalent surface represented by a node

In order to apply a heat flux to a node, an *equivalent* surface has to be defined for each node. In classical finite element method, this surface directly comes with the element structure whereas in a meshfree method, there is no such equivalent.

This equivalent surface has been chosen in the most natural way. In an axis-symmetric context, there are three main cases which are presented on figure 5.3:



**Figure 5.3:** Equivalent surface represented by a node



In the following, we are assuming that the initial node set is regular (as on figure 5.3 for example). Let us define the characteristic length  $h_a$  associated to a node  $\mathbf{x}_a$

$$h_a = \min \{ \|\mathbf{x}_a - \mathbf{x}_n\|_2 \mid n \in \llbracket 1; N \rrbracket \} \quad (5.6)$$

with  $N$  the total number of node. Since the node set is assumed to be regular,  $h_a$  is representing the length draw in red at nodes 1 and 2 on the figure 5.3.

As an axi-symmetric context is considered, the equivalent surface  $S_a$  must be calculated for an angle of 1 radian. The three cases described previously must be distinguished from each other in the calculation of the equivalent surface  $S_a$  represented by the node  $\mathbf{x}_a$ .

In the case n°1, the equivalent surface  $S_a$  is basically the external surface of a cylinder of height  $h_a$  and of radius  $r_a$  (with  $r_a = x_a$  the distance between the node  $\mathbf{x}_a$  and the axi-symmetric axis). Therefore:

$$S_a = r_a h_a \quad (5.7)$$

In the case n°2,  $S_a$  is the difference between the surface of a disc of radius  $r_a + \frac{h_a}{2}$  and a disc of radius  $r_a - \frac{h_a}{2}$ . Therefore:

$$\begin{aligned} S_a &= \frac{1}{2\Pi} \Pi \left( \left( r_a + \frac{h_a}{2} \right)^2 - \left( r_a - \frac{h_a}{2} \right)^2 \right) \\ &= r_a h_a \end{aligned} \quad (5.8)$$

In the case n°3,  $S_a$  should be divided in two parts:  $S_{a,\text{bottom}}$  and  $S_{a,\text{left}}$ :

$$\begin{aligned} S_a &= S_{a,\text{bottom}} + S_{a,\text{left}} \\ &= \frac{1}{2} \left( r_a h_a + \frac{h_a^2}{4} \right) + r_a \frac{h_a}{2} \\ &= r_a h_a + \frac{h_a^2}{8} \end{aligned} \quad (5.9)$$

In order to take this surface into account for the penalty force, it has been decided to replace the original penalty coefficient  $k$  (see equation 5.3) of dimension a stiffness ( $m^{-1}$ ) by a coefficient  $k_{S_a}$  of dimension a stiffness per surface unit ( $m^{-3}$ ). Thus, equation 5.3 becomes:

$$\mathbf{F}_{a,P} \cdot \mathbf{n} = \begin{cases} 0 & \text{if } g_a \leq 0 \\ -k_{S_a} \cdot S_a \cdot g_a & \text{if } g_a > 0 \end{cases} \quad (5.10)$$

Therefore, equation 5.4 about the potential energy becomes:

$$P_{a,p} = -\frac{1}{2}k_{S_a}S_a g_a^2 \quad (5.11)$$

Writing equation 5.11 assuming that the equivalent surface  $S_a$  represented by the node  $\mathbf{x}_a$  is constant over time is not physical. In reality, this assumption is false, especially in large deformation. However, in order to preserve the symmetry of the tangent matrix in the variational framework, this type of hypothesis is necessary but a compromised solution using the position of  $\mathbf{x}_a$  at the previous time step will be discussed in the next sections.

An other option is to not consider this surface at all but doing so amounts to ignore the node density in the surface area, which could lead to some errors.

### 2.3 Interdependence between friction and heating

The heat flux at the welding interface is directly related to the friction and contact. Indeed, the heat flux is proportional to the friction coefficient  $\nu$  and the gap  $g_a$ . We consider that the friction power  $P_{a,F}$  is completely transformed into heat power:

$$P_{a,F} = F_a^t v_a \quad (5.12)$$

where  $v_a$  is the velocity of the node  $x_a$  and  $\mathbf{T}_a$  is the tangent effort due to the Coulomb friction law given by:

$$F_a^t = \nu \|\mathbf{F}_{a,P}\|_2 = \nu F_{a,P}^n \quad (5.13)$$

The velocity of the node  $x_a$  is:

$$v_a = \omega \cdot r_a = \omega \cdot x_a \quad (5.14)$$

where  $\omega$  is the rotation speed. The same way a penalty force is applied on the considered node if the gap is negative, a heat flux is applied and is given by

$$\Phi = -\frac{1}{2} \frac{1}{T_0} \cdot \nu \cdot k_{S_a} S_a g_a \cdot \omega r_a \cdot dt \quad (5.15)$$

where  $\nu$  is the friction coefficient,  $r_a = x_a$  the radius of the considered node,  $k_{S_a}$  is the penalty stiffness per area unit,  $S_a$  is the equivalent surface represented by the node  $\mathbf{x}_a$  and  $dt$  is the current time step.

In this expression of  $\Phi$ , it is interesting to note that the factor  $\nu k_{S_a} S_a g_a$  is the tangent frictional force since a Coulomb frictional law is assumed and  $\omega r_a$  is the speed of the considered node. The factor  $\frac{1}{2}$  is due to the fact that the material is assumed to be the same in both parts. In the case of dissimilar materials with different effusivities  $b_1$  and  $b_2$ , this factor must be replaced by  $\frac{b_1}{b_1+b_2}$  [D'Alvise, 2002].

The heat flux can also be written

$$\Phi = -\frac{1}{2} \cdot \nu \cdot k_{S_a} \cdot \omega \cdot h \cdot dt \cdot (S_a(x_a) \cdot y_a \cdot x_a) \quad (5.16)$$

Some terms have to be added to the tangent matrix. However,

$$\frac{\partial \Phi}{\partial x} \neq 0 \text{ whereas } \frac{\partial F^t}{\partial T} = 0 \quad (5.17a)$$

$$\frac{\partial \Phi}{\partial y} \neq 0 \text{ whereas } \frac{\partial F^n}{\partial T} = 0 \quad (5.17b)$$

which is an issue in regard of the symmetry of the tangent matrix in the variational framework.

## 2.4 A symmetric formulation of frictional contact

The equation 5.17 shows that  $\frac{\partial \Phi}{\partial x} \neq \frac{\partial F^t}{\partial T}$  et  $\frac{\partial \Phi}{\partial y} \neq \frac{\partial F^n}{\partial T}$ . The *trick* we opt for is to use the position  $\mathbf{x}_a^{n-1}$  of the node  $\mathbf{x}_a$  at the previous time step  $n - 1$ . The algorithm is presented considering for one given node and a particular scenario:

1. time step n-2:  $g \geq 0$ , no penetration.
2. time step n-1:  $g < 0$ , penetration detected:
  - (a) Save of the current coordinates  $\mathbf{x}_a^{n-1}$
  - (b) Computation of the penalty force  $\mathbf{F}^{n-1} = -k_{S_a} y^{n-1} S_a(x^{n-2})$ .
  - (c) Apply the penalty force  $\mathbf{F}^{n-1}$ .
  - (d) Add  $\frac{\partial^2 P_F^{n-1}}{\partial y^2}$  to the stiffness matrix.
3. time step n:  $g < 0$ , penetration detected:
  - (a) Computation of the heat flux due to previous time step:  
 $\Phi^n = -\frac{1}{2} \cdot \nu \cdot k \cdot \omega \cdot h \cdot dt \cdot S_a^{n-1} y_a^{n-1} x_a^{n-1}$
  - (b) Apply the heat flux due to previous time step  $\Phi^n$ .
  - (c) Save of the current coordinates  $\mathbf{x}_a^n$
  - (d) Computation of the penalty force  $\mathbf{F}^n = -k_{S_a} y^n S_a(x^{n-1})$ .
  - (e) Apply the penalty force  $\mathbf{F}^n$ .
  - (f) Add  $\frac{\partial^2 P_F^n}{\partial y^2}$  to the stiffness matrix.
4. time step n+1:  $g \geq 0$ , no penetration.

- (a) Computation of the heat flux due to previous time step:

$$\Phi^{n+1} = -\frac{1}{2} \nu k_{S_a} \omega h dt S_a^n y_a^n x_a^n$$

- (b) Apply the heat flux due to previous time step  $\Phi^{n+1}$ .

5. etc.

Thanks to this *trick*,  $\Phi$  is now independent of  $\mathbf{x}$ , such that

$$\frac{\partial^{n+1}\Phi}{\partial x^{n+1}} = \frac{\partial^{n+1}\Phi}{\partial y^{n+1}} = 0 \quad (5.18)$$

Similarly, the potential energy due to the penalty force  $P_{a,P}$  is now independent of  $r_a = x_a$  (see equation 5.11).

The symmetry of the tangent matrix is now respected. The price to pay for using this algorithm is that the heat flux is applied one time step too late. However, if the time step is small enough (which will typically be the case for this type of problem), this error seems reasonable.

### 3 Identification of the flow stress models

Series of experiments have been lead by ACB, a partner of the consortium. These experiments had three objectives: to characterize a weld made with the RFW process, propose industrial tests and to be a support for the validation of our LME interpolation approach implementation.

A total of twenty-six welds have been done using a p265gh steel, which is a material mainly used to build pressure vessels. This series of tests was made by tuning the different welding parameters which are the expected total upset, the rotation speed, the welding force and the forging force.

The LAMPA institute identified two flow stress models for the p265gh steel: the Norton-Hoff model and the Johnson-Cook model.

#### 3.1 Norton-Hoff model

The Norton-Hoff flow stress model has been identified for two temperature domains:  $T < 600$  °C and  $T \geq 600$  °C (a solid-solid ophase transformation occurs around 600°C). Let us remind that the Norton-Hoff law is defined by

$$\sigma_y(\dot{p}, T) = A \exp\left(\frac{B}{T}\right) \left(\frac{\dot{p}}{\dot{p}_0}\right)^{1/m} \left(\frac{p}{p_0}\right)^{1/n} \quad (5.19)$$

where  $A$ ,  $B$ ,  $m$  and  $n$  are material constants. For each domain, the parameters have been respectively identified at  $T = 300$  °C and  $T = 600$  °C. The results are shown in table 5.1.

Parameters	$\sigma_y(\dot{p}, T) = A \exp\left(\frac{B}{T}\right) \left(\frac{\dot{p}}{\dot{p}_0}\right)^{1/m} \left(\frac{p}{p_0}\right)^{1/n}$	
	$T < 600 \text{ }^\circ\text{C}$	$T \geq 600 \text{ }^\circ\text{C}$
$A$ (MPa)	401	3.2
$B$ (K)	242	4430
$m$	0.02	0.14
$n$	0.10	0.09
$\dot{p}_0$ ( $\text{s}^{-1}$ )	0.001	0.001
$p_0$	0.0002	0.0002

**Table 5.1:** Identification of the Norton-Hoff law parameters for p265gh steel.

Parameters	$\sigma_y(p, \dot{p}, T) = \left[A + Bp^{\frac{1}{n}}\right] \left[1 + C \ln\left(\frac{\dot{p}}{\dot{p}_0}\right)\right] \left[1 - \left(\frac{T - T_0}{T_m - T_0}\right)^{\frac{1}{m}}\right]$	
	$T < 600 \text{ }^\circ\text{C}$	$T \geq 600 \text{ }^\circ\text{C}$
$\dot{p}_0$	0.05	0.05
$T_0$ ( $^\circ\text{C}$ )	300	600
$T_m$ ( $^\circ\text{C}$ )	1500	1500
$A$ (MPa)	330	125
$B$	280	200
$n$	2.632	10
$C$	0	0.18
$m$	0.714	2.778

**Table 5.2:** Identification of the Johnson-Cook law parameters for p265gh steel.

### 3.2 Johnson-Cook model

The Johnson-Cook flow stress model has been identified for two temperature domains:  $T < 600 \text{ }^\circ\text{C}$  and  $T \geq 600 \text{ }^\circ\text{C}$ . Let us remind that the Johnson-Cook law is defined by

$$\sigma_y(p, \dot{p}, T) = \left[A + Bp^{\frac{1}{n}}\right] \left[1 + C \ln\left(\frac{\dot{p}}{\dot{p}_0}\right)\right] \left[1 - \left(\frac{T - T_0}{T_m - T_0}\right)^{\frac{1}{m}}\right] \quad (5.20)$$

where  $p_p$  is the equivalent plastic strain,  $\dot{p}_p$  is the plastic strain rate and  $A, B, C, n$  and  $m$  are material constants. For each domain, the parameters have been respectively identified at  $T = 300 \text{ }^\circ\text{C}$  and  $T = 600 \text{ }^\circ\text{C}$ . The results are shown in table 5.2.

## 4 RFW process modeling

### 4.1 Boundary conditions and loading

The simulation parameters are the one described previously: axi-symmetric context, quasi-static computation, the contact surface is assumed to be a plane and a Coulomb friction law is considered. The flow stress model is Johnson-Cook model as identified on table 5.2.

In this section a hollow cylinder is considered. The global geometry of the part is described on figure 5.4. A bore is made at the bottom of the part such that the cylinder is not totally hollow. The real loading is applied on the top of the part (black arrows). In this simulation, only the reduced thickness portion is considered (blue part). Therefore, additional hypothesis are made, assuming the hypotheses made at the beginning of the chapter:

- The loading is half the real loading (blue arrows).
- The degree of freedom related to the vertical displacement of the node on the top is the same for each node (red nodes).

The boundary conditions are presented on figure 5.1. The initial node-set is given on figure 5.5. The loading given on figure 5.6 is applied on the nodes on the top of the node-set. A delay between the experimental and the numerical loading is applied in order to facilitate the convergence.

### 4.2 Results of the modeling.

Figures 5.7 shows the state of the welding at successive time steps. The first thing to note is that the simulation currently stops converging after 10.6 s so it is only possible at the moment to study the results with this first half of the process.

At  $t = 0.2$  s (see figure 5.7-a and 5.8), the temperature field has a cone shape as the radius  $r_a = x_a$  increases. This result is physical since the outer nodes have a higher velocity than the inner ones. Therefore, the heat flux is larger for outer nodes (see equation 5.16).

As the time passes, nodes at the center of the modeled part progressively become the warmer nodes as shown by figures 5.7-b and 5.7-c . Indeed, because of the loading, nodes at the bottom corners are no more in contact with the contact surface so no heat flux is applied whereas it is still the case at the center.

From this moment, the flash starts to appear on each side and the temperature at the center of the part is quickly increasing. The early flash formation is in agreement (at least geometrically) with other numerical result from the literature using the FEM [D'Alvise, 2002]: figure 5.7 shows that the largely deformed area is

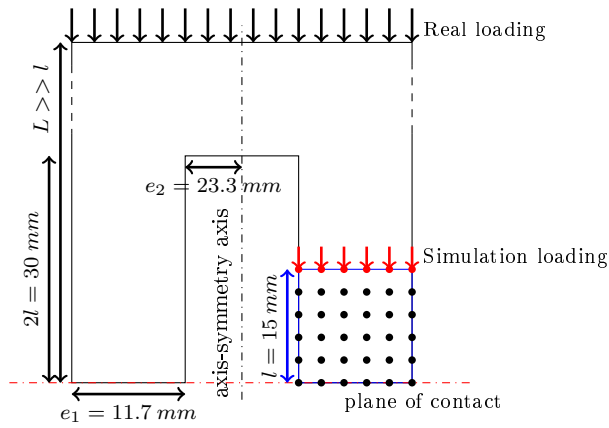


Figure 5.4: Initial node-set for RFW modeling.

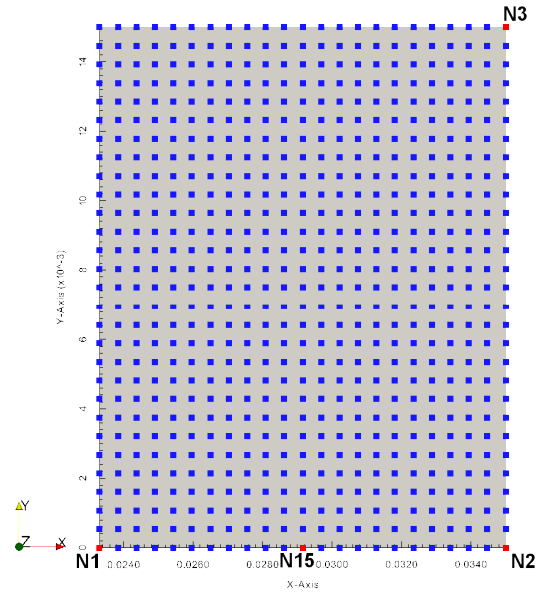


Figure 5.5: Initial node-set for RFW modeling.

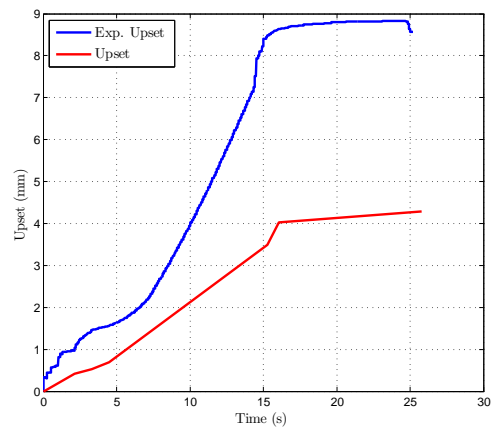
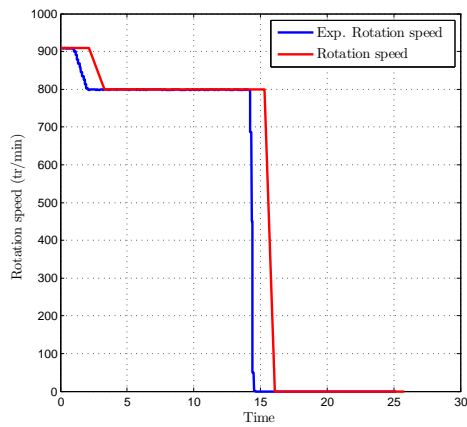


Figure 5.6: Rotary speed and applied force.

only at the bottom of the part while the upper part remains undeformed. Because of the hypothesis presented on figure 5.4 to just model a part of the hollow cylinder, a global average displacement of  $0.8\text{ mm}$  is observed on the x-direction.

Figure 5.10 shows the final configuration at the end of the welding. The asymmetry between the inner and outer sides of the part is also visible with the simulation on figure 5.11. The Heat Affected Zone (HAZ) on figure 5.10 is located in the largely deformed area. Numerically, this same area is also the hottest so the prediction of the HAZ seems to be in agreement with the experimental data.

The figure 5.12 shows the evolution of the temperature for four selected nodes depicted on figure 5.5: the outer node N3 of the top of the modeled area, the end nodes N1 and N2 of the flash and the mid node of the contact area N15. The melting temperature used for the Johnson-Cook model is also represented. At the beginning, the outer node heats up faster because of the rotary speed. At  $t \simeq 3\text{ s}$ , the end node N2 of the flash separates from the contact plane and N15 becomes the hottest node. At  $t \simeq 8.5\text{ s}$ , the temperature of node N15 becomes greater than the melting temperature of the material. In reality, this phenomena should not happen: the temperature is supposed to be very close but still lesser than the melting temperature. This may be a first cause of the non-convergence of the calculation.

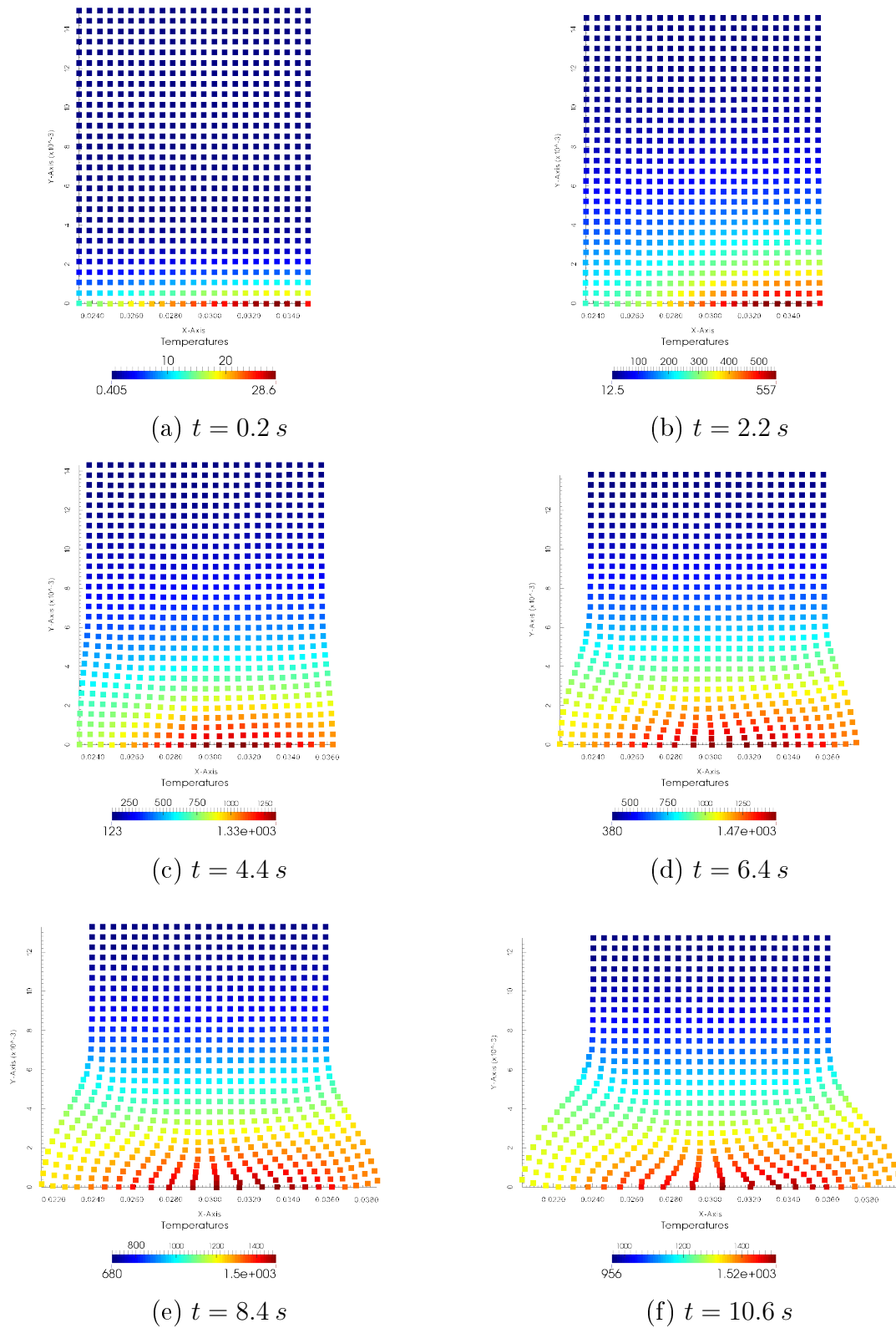
An other cause for the non convergence may be the flow stress model used. Even if Johnson-Cook is one of the most used model, it may be not sophisticated enough for the simulation of a very numerical demanding problem.

Finally, the reason most likely to cause the non-convergence may be the contact friction law. The penalty method is very simple and since the modeling of the heat flux is critical for the modeling of the process, it may be not accurate enough. Taking the convection phenomena may also solve the issue of temperature greater than the melting temperature: it may be used the whole hollow cylinder and the cooling to the ambient air.

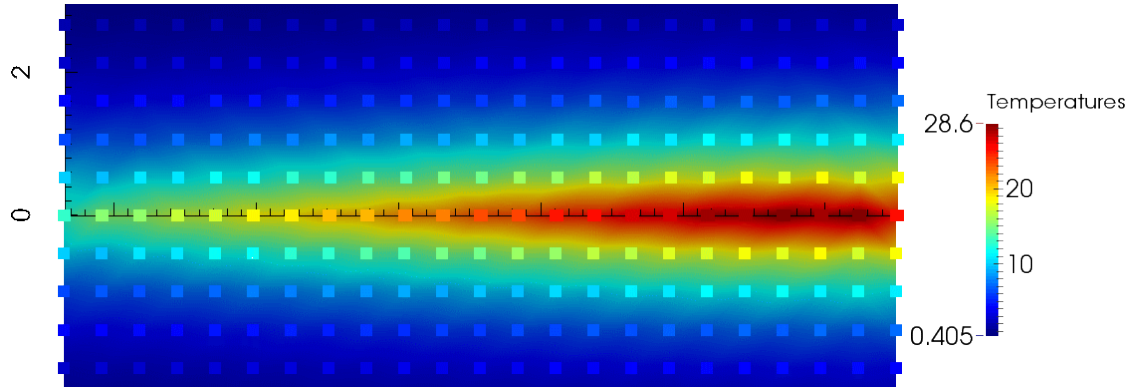
Different penalty coefficient (see equation 4.2) have been tested for the simulation of the process. A comparison of the results is available in table 5.3. The results presented previously are the one obtained with  $C = 10\text{ m}$  since it is considered as a good compromise between accuracy and calculation time. However, the results between the three cases are very close from each other.

Different node-sets have also been tested: the represented one, a twice as dense node set and a final one with more nodes at the bottom. The very dense node set was unusable because of the computation time. The locally refined node set gave bad results especially because of lack of regularity of the node-set dependency of our implementation (see the patch test in the previous chapter). A compromise between node density and computation time lead to the node-set presented on figure 5.5.

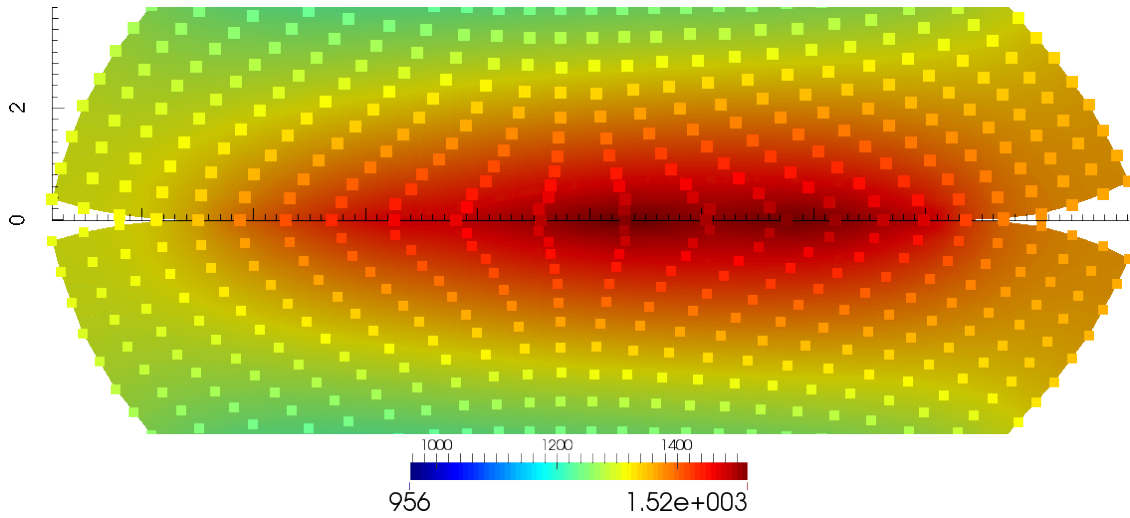




**Figure 5.7:** Evolution of the welding modeling.



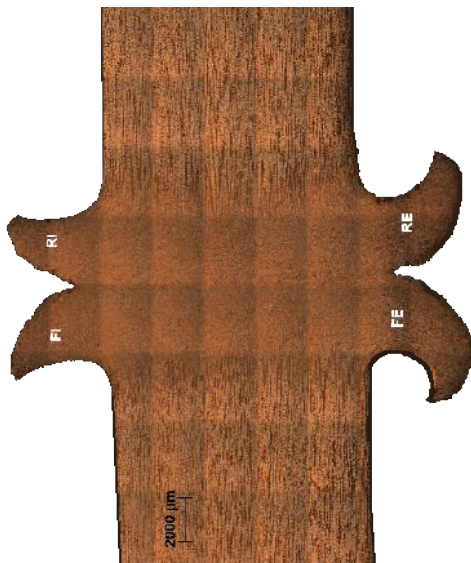
**Figure 5.8:** Displacement and temperature field at  $t = 0.2$  s.



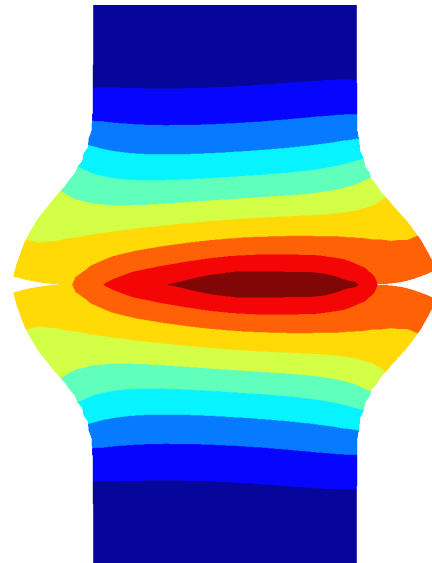
**Figure 5.9:** Displacement and temperature field at  $t = 10.6$  s.

$C$ (m)	1	10	100
Max. penetration ( $\mu m$ )	58.8	5.633	5.240
Max. temperature (K)	1516.37	1517.37	1517.47
Total width (mm)	7.858	7.532	7.530
Computation duration	$\simeq$ 1h40min	$\simeq$ 7h20min	$\simeq$ 10h10min
Time modeled before divergence (s)	11.004	10.63	10.51

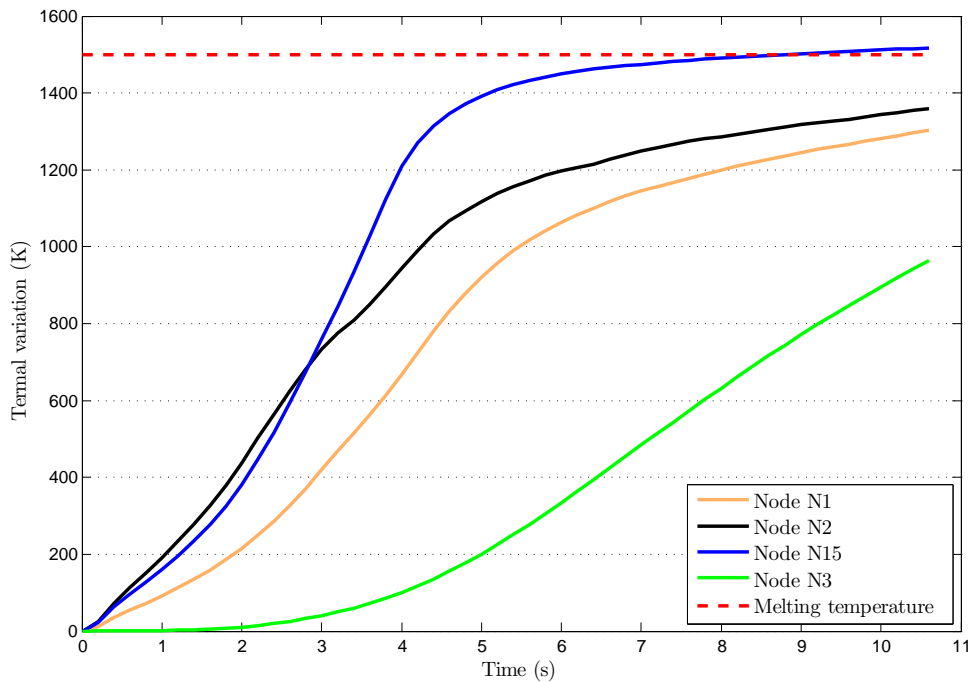
**Table 5.3:** Influence of the penalty coefficient on the RFW modeling



**Figure 5.10:** Experimental Heat Affected Zone (HAZ).



**Figure 5.11:** Continuous view of the last time step simulated ( $t = 10.6 \text{ s}$ ). Color map represents the temperature.



**Figure 5.12:** Evolution of the temperature for different nodes.

## 5 Conclusion

Our LME implementation seems to be able to simulate the RFW process. The LME approach presents the necessary features such as the possibility to put contact conditions and thermal fluxes related to these conditions. A notion of equivalent surface represented by a node but also a symmetric formulation of frictional contact have been added.

The first results of the modeling of the RFW process are promising. Even if currently, the implementation may not be robust enough to simulate the whole process, the first results are close to the experimental data on the first half of the process and the evolution of the flash shape is in agreement with literature and experimental observations.

Different way are possible to improve the convergence. The contact modeling may be improve, by using an augmented Lagrangian model for instance. A more accurate flow stress model could be use to improve the behavior of the material. The convection phenomena should also be taken into account. This should solve the fact that for some nodes, the temperature reaches the melting temperature.

Finally, it is important to note that phase change has not been taken into account.



# Conclusion

In this work, a meshless method based on the Local Maximum Entropy (LME) principle and using material point integrations is proposed.

Our work is mainly focused on simulations of general solid flows involving coupled thermo-mechanics, large deformations and transient loading. For this purpose, the meshless methods are an excellent alternative to conventional grid-based methods. Our implementation is a Lagrangian particle method, which preserves all the advantages of absence of a mesh. Furthermore, by employing the LME approximation scheme, it overcomes the difficulty for imposition of essential boundary conditions encountered in most of the meshless methods.

This work is also using an incremental variational approach. This variational framework allows the description of the state and the evolution of a boundary values problem. It includes inelasticity, equation of state, and general geometries in  $\mathbb{R}^d$  and boundary conditions. The incremental aspect leads to a temporal discretization of the governing equation to reduce time-dependent problems to a sequence of incremental problems each characterized by a minimum principle. Then, a spatial discretization is applied to describe locally the state of the material via a Rayleigh-Ritz approach. The material points are a support of this discretization since they are tracking the material and are carrying all the material information, including the inner variables. Then a fully discrete formulation is obtained. From this, the evolution of the solid flows can be solved forward explicitly or implicitly.

The possibilities of this implementation have been described and verified through some test cases in chapter 4 by being compared to equivalent simulations using finite elements method. In particular, conduction and purely mechanical problems can be treated with a high accuracy. Moreover, the possibility to apply a contact condition allows to model more complex problems.

An other drawback of the current implementation is the computation time required when the number of node is rising. An improvement would be to use a skyline matrix storage (SKS) to ease the inversion of matrices when using an implicit scheme. An other way would be to use an explicit scheme "à la" OTM but implementing conduction and thermo-mechanical coupling are real issues.

However, the implementation seems to have a high dependency on the node-set in the computation of the shape functions, mostly because of the very local  $h$  parameter we chose. In the patch test, the differences on the accuracy between a very regular node set and an irregular one are very important. This fact does not allow us to perform an efficient updated Lagrangian approach.

The first results obtained with the simulation process are very encouraging but are not satisfying enough to be industrially used. The heat flux at the interface represents a very critical part of the process and is highly depending on the contact modeling. Since only a very basic penalty contact model has been implemented, the heat flux may not be very accurate. An improvement of the contact model, such as augmented Lagrangian contact, may improve drastically the accuracy of the temperature field and therefore the global result of the simulation. As shown in chapter 5, the formation of the flash in the simulation is only starting to appear. Using an updated Lagrangian approach by updating the neighborhoods of the material points may improve the formation of the flash. In the same way, the use of more complex flow stress models may improve the results of the RFW modeling.

The positive aspects of the current results are that the formation of the flash is in agreement with what can be found in literature and that the temperature at the interface is very close to the melting temperature of the material. A solution to avoid that the temperature becomes greater than the melting temperature for some nodes would be to consider some convection condition at the boundary of the domain, which could represent the thermal exchanges with the ambient air and the thermal conduction with a non modeled part of the hollow cylinder.

# Appendix A

## Update of the neighborhoods

The update of neighborhoods enables a very interesting aspect of the maximum entropy interpolation: the ability to automatically manage self-contact by considering it as a sticking contact.

Figure A.1 represents a case of self contact with the red-colored part coming from the right. Assuming that the red-colored part is the neighborhood of the material point represented here by a blue circle, consider two cases: the first one without update of neighborhood and a second one with it.

In the first case, the shape functions are computed in the initial configuration so the nodes of the main part are not in the neighborhood of the considered material point. Therefore, the red part will just go through the main one.

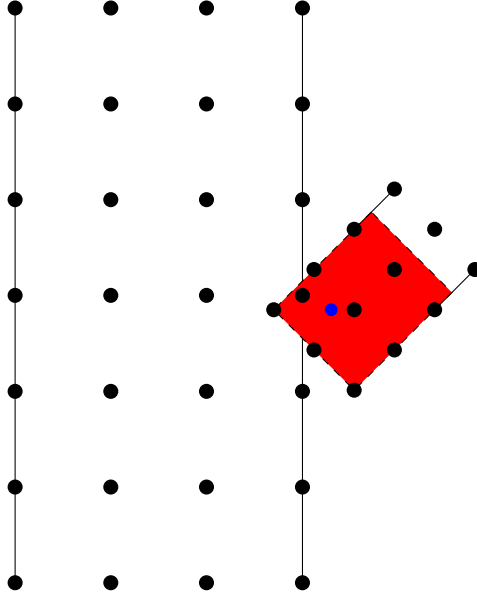
In the second case, as the material point is coming closer to the main part, the closest node of the main part will be added to the material point neighborhood. Therefore, all global and internal variables will homogenize in this neighborhood and it will numerically be impossible to dissociate those two parts during the remainder of the simulation.

Independently of the MaxEnt interpolation, there are three ways of performing a simulation: with a total Lagrangian approach, an updated Lagrangian approach and the Eulerian approach. Here, a (total or updated) Lagrangian approach is used.

First of all, given an initial node-set, every single material point is created. Then every shape function is evaluated at every material points. Depending on those values, the original neighborhood of each material point is built: for instance, a node  $x_a$  is in the neighborhood of a material point  $x$  if, and only if, shape function evaluation  $p_{\beta_a}(x)$  is greater than a tolerance parameter which depends on  $h$  and  $\gamma$ . With a total Lagrangian approach, everything is computed in this initial setup.

On the contrary, in an updated Lagrangian approach, the shape functions and the neighborhood are computed at each time step; or at least when the deformation gets too large. But in this case, the  $h$  parameter should be updated, which implies either to define a metric of the deformation or to rebuild a connectivity table in order to find the  $h$  in the shape functions computations. The first case scenario may be inaccurate and the second one consists in re-meshing the whole node-set, which





**Figure A.1:** Self contact between two initially distant part of a same mechanical part.

is basically what is tried to be avoided. An original solution consists in bringing back the deformed system in a pseudo-initial state.

At each time step  $n + 1$ , the following procedure is used: first the updated positions  $X_{up}$  of each node are computed. Then the updated positions of each material point are also computed from the new position of each node in his former neighborhood. In the same way, a strain gradient  $F_{mPt}$  is interpolated for each material point:

$$F_{mPt} = \sum_{a=1}^n \nabla p_a(x) \mathbf{x}_a + \mathbf{I} \quad (\text{A.1})$$

where  $n$  is the size of the material point neighborhood  $N_n$ . The strategy to compute the shape function is to use the pseudo-initial coordinates of each nodes and material points defined by:

$$X'_0 = F_{mPt}^{-1} X_{up} \quad (\text{A.2})$$

In this case, the  $h$  parameter is still adapted and updated neighborhoods and shape functions can be computed.

The update of the neighborhood is very interesting to use when the convex domain  $convX$  becomes non convex during the simulation: it basically consists in cutting the whole domain in smaller convex domains. In case of very large deformation in traction for instance, the update of the neighborhood also allows for a material

point to catch only the most relevant information.

Let us consider a current time step  $n$  where the position of the nodes  $\mathbf{x}_a^n$ , the position of the material points  $\mathbf{x}_{mPt}^n$  and the gradient of deformation  $\mathbf{F}_{0 \rightarrow n}$  are assumed to be known. The objective is to determine  $\mathbf{F}_{0 \rightarrow n+1}$ ,  $\mathbf{x}_a^{n+1}$  and  $\mathbf{x}_{mPt}^{n+1}$  by introducing the pseudo-initial state. Let us consider one material point.

$$\mathbf{F}_{0 \rightarrow n+1} = \mathbf{F}_{n \rightarrow n+1} \cdot \mathbf{F}_{0 \rightarrow n} \quad (\text{A.3})$$

The objective is reduced to the computation of  $\mathbf{F}_{n \rightarrow n+1}$ . By definition:

$$\begin{aligned} \mathbf{F}_{n \rightarrow n+1} &= \frac{\partial \mathbf{u}_{n \rightarrow n+1}}{\partial \mathbf{X}_n} + \mathbf{I} \\ &= \sum_a \frac{\partial p_a}{\partial \mathbf{x}_a} \mathbf{u}_a^{n \rightarrow n+1} + \mathbf{I} \end{aligned} \quad (\text{A.4})$$

where:

$$\begin{aligned} \frac{\partial p_a}{\partial \mathbf{x}_a} &= \frac{\partial p_a}{\partial \mathbf{x}'_a} \frac{\partial \mathbf{x}'_a}{\partial \mathbf{x}_a} \\ &= \frac{\partial p_a}{\partial \mathbf{x}'_a} \mathbf{F}_{0' \rightarrow n}^{-1} \\ &= \frac{\partial p_a}{\partial \mathbf{x}'_a} \mathbf{F}_{0 \rightarrow n}^{-1} \end{aligned} \quad (\text{A.5})$$

Then, equation A.3 becomes:

$$\begin{aligned} \mathbf{F}_{0 \rightarrow n+1} &= \left( \sum_a \frac{\partial p_a}{\partial \mathbf{x}'_a} \mathbf{F}_{0 \rightarrow n}^{-1} \mathbf{u}_a^{n \rightarrow n+1} + \mathbf{I} \right) \cdot \mathbf{F}_{0 \rightarrow n} \\ &= \sum_a \frac{\partial p_a}{\partial \mathbf{x}'_a} \mathbf{u}_a^{n \rightarrow n+1} + \mathbf{F}_{0 \rightarrow n} \end{aligned} \quad (\text{A.6})$$

where  $p_a$  are computed in the pseudo-initial configuration. The update of the material point coordinates is given by:

$$\mathbf{x}_{mPt}^{n+1} = \sum_a p_a(\mathbf{x}_a) \mathbf{u}_a^{n \rightarrow n+1} + \mathbf{x}_{mPt}^n \quad (\text{A.7})$$

In the cases where the neighborhoods are updated at each time step, the algorithm is the following:

1. Beginning of time step  $n + 1$ .

2. Update of the nodes coordinates:  $\mathbf{x}_a^{n+1} = \mathbf{x}_a^n + \mathbf{u}_a^{0 \rightarrow n+1}$
3. Update of the neighborhoods in the pseudo-initial configuration.
4. Computation of the shape functions and its derivatives in the pseudo-initial configuration.
5. Computation of the deformation gradient.
6. Update of the material point coordinates.

In the current implementation, the calculation of  $\boldsymbol{\lambda}^*$  becomes very difficult as soon as the node-set is highly irregular (i.e. two nodes become very close from each other). Because of that, no practical case is presented in this document.

# Résumé du chapitre 1

## Introduction

### 1 Procédé de soudage par friction rotative à entraînement direct (RFW)

Cette thèse s'inscrit dans un projet industriel dont l'objectif est le développement d'une machine de soudage par friction rotative capable de souder des pièces de grande taille. Ce projet implique deux entreprises, ACB [<http://www.acb-ps.com>] et Jeumont Electric [<http://www.jeumontelectric.com>], ainsi que trois laboratoires, le GeM (Institut de recherche en Génie civil et Mécanique), l'IMN (Institut des matériaux de Nantes) et le LAMPA (Laboratoire des Arts et Métiers ParisTech d'Angers).

Dans ce contexte, une méthode sans maillage basée sur une interpolation de type Local Maximum Entropy (LME) est proposée pour modéliser des phénomènes de fort couplage thermomécanique mettant en jeu de grandes déformations et des conditions limites de contact. Ce type d'approche évite les problèmes inhérents aux étapes de remaillage indispensables en grandes déformations avec une approche de type Méthode des Éléments Finis (MEF) qui découle inévitablement à une dégradation des différents champs tels que le champ de température par exemple. Ce dernier point est crucial dans le cas de la modélisation du procédé RFW.

Le procédé RFW est maintenant utilisé industriellement depuis plusieurs décennies et permet le soudage de deux pièces n'étant pas nécessairement faites du même matériau. Les différentes étapes de soudage sont la mise en mouvement, la phase d'approche, le soudage et la forge.

La modélisation du procédé RFW par la MEF est maintenant relativement maîtrisée. Les codes commerciaux, comme Abaqus [[Simulia Abaqus, 2010](#)] par exemple, proposent les scripts nécessaires pour simuler "facilement" le procédé.

Les résultats obtenus avec la MEF sont suffisamment proches des résultats expérimentaux pour être considérés comme fiables. Cependant la MEF n'est peut-être pas la méthode la plus adaptée à la modélisation du procédé. En effet, le couple thermo-mécanique joue un rôle très important et requiert une grande précision sur les différents champs. Pour prendre en charge des grandes déformations, les modélisations par éléments finis procèdent à des étapes de remaillage. Des études montrent que généralement, des maillages adaptatifs donnent des prédictions raisonnables du résultat mais peuvent manquer totalement des phénomènes physiques tels que les bandes de cisaillement adiabatique [Lee and Bathe, 1994]. Une précision insuffisante du champ de température peut poser des problèmes pour d'éventuelles études métallurgiques par la suite.

## 2 Objectifs

Le procédé RFW est contrôlé en manipulant trois paramètres: la vitesse de rotation, le couple et la pression appliquée sur la pièce en rotation. Effectuer des campagnes d'essais visant à déterminer les paramètres de soudage optimaux est efficace mais requiert énormément de temps et d'investissement.

Les modélisations de type MEF sont efficaces mais la nécessité de procéder à de nombreuses phases de remaillage représente à la fois une perte de temps et de précision due au transfert des différents champs de l'ancien maillage vers le nouveau.

Quant à elles, les approches de type sans maillage présentent de nombreux avantages vis-à-vis des méthodes basées sur une grille telle que la MEF. L'argument le plus pertinent dans le contexte considéré est certainement que ce type de méthode ne souffre pas des grandes déformations. Les autres grands avantages sont la grande régularité des fonctions d'interpolation, une convergence généralement meilleure et la possibilité d'éviter le verrouillage volumétrique en réglant le paramètre de dilatation de la fonction noyau.

Le but de cette étude est de développer et de proposer une méthode sans maillage basée sur une approche de type Local Maximum Entropy (LME). Les objectifs sont d'être capable de prédire le champ de température, la zone affectée thermiquement ainsi que le champs de déformation.

Les approches LME ne sont utilisées que depuis quelques années dans l'industrie et fournissent déjà des résultats très impressionnants, notamment dans la modélisation d'impacts à grande vitesse via la Optimal Transportation Method de Li. et al [Li et al., 2010]. Une approche LME devrait donc être capable de fournir de bons résultats pour la modélisation du procédé RFW.

## 3 Méthodes d'approximation sans maillage

Au cours des dernières décennies, une multitude de méthodes sans maillages a été

publiée. Cependant, toutes présentent un grand nombre de similarités entre elles et la principale différence réside dans la façon de construire l'approximation  $u^h(\mathbf{x})$  d'une fonction  $u(\mathbf{x})$  dont le domaine de définition est fixé par un nuage de nœuds pré-défini. Une telle approximation peut toujours s'écrire sous la forme continue

$$u^h(\mathbf{y}) = \int_{\Omega} C\Phi(\mathbf{y})u(\mathbf{y}) d\mathbf{y}$$

ou sous la forme discrète

$$u^h(\mathbf{x}) = \sum_{a=1}^n N_a(\mathbf{x})u(\mathbf{x}_a)$$

où  $\Phi$  est la fonction noyau,  $C$  un coefficient (de normalisation par exemple) et  $N_a$  la fonction d'interpolation associée au nœud  $a$ .

Cependant, le principal défaut des méthodes sans maillages les plus connues telles que Smooth Particule Hydrodynamics (SPH) [Lucy, 1977], Moving Least Squares (MLS) [Lancaster and Salkauskas, 1981] [Nayroles et al., 1992] ou encore Reproducing Kernel Particule Method (RKPM) [Liu et al., 1995] est l'impossibilité de définir des conditions limites précises simplement. Dans cette optique, la Natural Element Method (NEM) [Sukumar, 1998] [Sukumar and Moran, 1999] [Sukumar et al., 2001] est le premier véritable succès. La NEM est même utilisable pour la modélisation des procédés [Chinesta et al., 2013].

La Material Points Method (MPM) [Sulsky et al., 1993] [Sulsky et al., 1995] est une des bases de notre travail. La particularité de cette méthode est d'être capable de prendre en compte la dépendance historique des lois constitutives dans une large gamme de problèmes d'ingénierie tels que les problèmes de pénétration ou d'impact. Elle consiste en une procédure de discrétisation généralisée en utilisant une approche variationnelle et un schéma de discrétisation de type Petrov-Galerkin, l'idée principale étant de pouvoir profiter à la fois des avantages des méthodes eulériennes et lagrangiennes.

L'objectif de ce travail de thèse consiste donc à construire une méthode sans maillage basée sur une approche Local Maximum Entropy (LME) combinée à une formulation variationnelle prenant en compte un fort couplage thermo-mécanique. Ceci passera donc par la modélisation variationnelle basée sur une approche énergétique incrémentale puis par la construction des fonctions d'interpolations LME. Cette implémentation sera ensuite testée sur des cas tests classiques pour finalement être utilisée pour modéliser le procédé de soudage par friction rotative.



# Résumé du chapitre 2

## Formulation du problème thermo-mécanique

### 1 Méthodes variationnelles basées sur une approche énergétique et principes variationnels

Les principes variationnels jouent un rôle important en mécanique depuis plusieurs décennies [Lanczos, 1970] et ont principalement été développés pour des systèmes conservatifs. Les plus connus sont certainement les principes de Hamilton [Hamilton, 1834] en dynamique et de minimisation de l'énergie potentielle en statique. D'un point de vue mathématique mais aussi mécanique, les approches variationnelles présentent de nombreux avantages tels que l'unicité, la convergence et la stabilité des formulations. Fondamentalement, un principe variationnel se résume à une approche optimisation utilisée pour décrire l'état et l'évolution d'un problème aux conditions limites.

Soit  $\phi(t)$  l'état du système au temps  $t$ . On cherche à définir un principe variationnel déterminant l'évolution du système considéré, dans un cadre dynamique ou quasi-statique.

Le principe de Hu-Washizu-Fraeijs de Veubeke [Washizu, 1955] [de Veubeke, 1972] [Hu, 1984] est le principe canonique de l'élasticité en statique. Il met en jeu trois champs indépendants que sont: la configuration  $\phi$ , le gradient de déformation  $\mathbf{F}$  et le tenseur des contraintes de Piola  $\mathbf{P}$ . La fonctionnelle associée est définie par:

$$\mathcal{H}(\phi, \mathbf{F}, \mathbf{P}) = \int_{B_0} [W(\mathbf{F}^T \mathbf{F}) + \mathbf{P} \cdot (\nabla \phi - \mathbf{F})] dV - \int_{B_0} \rho_0 \bar{\mathbf{b}} \cdot \phi dV - \int_{\partial_\sigma B_0} \bar{\mathbf{t}} \cdot \phi dA$$

où  $W$  est l'énergie de déformation élastique propre dépendant du tenseur de Cauchy  $\mathbf{C} = \mathbf{F}^T \mathbf{F}$ ,  $\bar{\mathbf{b}}$  est la résultante des forces volumiques et  $\bar{\mathbf{t}}$  les efforts appliqués à la



surface  $\partial_\sigma B_0$ . Les trois dérivées de Gâteaux mènent respectivement à l'équation d'équilibre, l'équation constitutive et l'équation de compatibilité:

$$\begin{aligned}\nabla \cdot \mathbf{P}^T + \rho_0 \bar{\mathbf{b}} &= 0 \\ \mathbf{P} &= 2\mathbf{F} \frac{\partial W(\mathbf{C})}{\partial \mathbf{C}} \\ \mathbf{F} &= \nabla \phi\end{aligned}$$

Considérons maintenant le principe de minimum d'énergie potentielle et construisons une modélisation variationnelle. Si les équations de compatibilité et constitutives sont vérifiées, alors la fonctionnelle devient

$$\begin{aligned}\mathcal{H}(\phi) &= \int_{B_0} W(\nabla_0 \phi^T \cdot \nabla_0 \phi) dV - \int_{B_0} \rho_0 \bar{\mathbf{b}} \cdot \phi dV - \int_{\partial_\sigma B_0} \bar{\mathbf{t}} \cdot \phi dA \\ &= \mathcal{U}(\phi) - \mathcal{W}(\phi)\end{aligned}$$

avec  $\mathcal{U}(\phi)$  l'énergie potentielle et  $\mathcal{W}(\phi)$  l'énergie des forces extérieures et où  $\rho_0$  est la masse volumique et  $\mathbf{grad}_0$  le gradient matériel. Le problème aux conditions limites peut donc être décrit par le principe variationnel suivant:

$$\inf_{\phi \text{ adm.}} (\mathcal{U}(\phi) - \mathcal{W}(\phi))$$

Dans ce contexte, les équations d'équilibre sont données par:

- Conservation de la masse

$$\rho \det \mathbf{F} = \rho_0$$

où  $\rho$  est la masse volumique dans la configuration déformée.

- Conservation du moment linéaire

$$\rho_0 \ddot{\phi} = \nabla_0 \cdot \mathbf{P}^T + \rho \mathbf{b}$$

- Conservation du moment angulaire

$$\mathbf{P}\mathbf{F}^T = \mathbf{F}\mathbf{P}^T$$

Le couplage thermo-mécanique impliquent deux lois de conservation supplémentaire représentant les lois de la thermo-dynamique:

- Conservation de l'énergie (premier principe de la thermo-dynamique)

$$\rho_0 T \dot{\eta} = \mathbf{P}^d \cdot \dot{\mathbf{F}} + \mathbf{Y}^d \cdot \dot{\mathbf{Z}} - \nabla_0 \cdot \mathbf{H} + \rho_0 Q$$

où  $\mathbf{H}$  est le vecteur flux de chaleur nominal,  $Q$  le flux de chaleur massique appliqué et  $\eta$  l'entropie massique.

- Inégalité de Clausius-Duhem (second principe de la thermo-dynamique)

$$T\dot{\Gamma} = \mathcal{D}_{\text{int}} - \frac{1}{T}\mathbf{H} \cdot \nabla_0 T \geq 0$$

où  $T$  est la température absolue et  $\dot{\Gamma}$  le taux de production d'entropie.

## 2 Cadre général en thermo-mécanique

Considérons un problème aux conditions limites avec un fort couplage thermo-mécanique. Les équations constitutives en grandes déformations sont données localement par [Yang et al., 2006]

$$\begin{aligned}\nabla \cdot \mathbf{P} + \rho_0 \mathbf{B} &= \rho_0 \dot{\mathbf{V}} \\ \mathbf{F} \mathbf{P}^T &= \mathbf{P} \mathbf{F}^T \\ \dot{E} &= \mathbf{P} \cdot \dot{\mathbf{F}} + \rho_0 Q - \nabla \cdot \mathbf{H} \\ \dot{\gamma} &\equiv \rho_0 \dot{\eta} - \frac{\rho_0 Q}{T} + \nabla \cdot \frac{\mathbf{H}}{T} \geq 0\end{aligned}$$

On peut définir un pseudo-potetiel de dissipation  $\Delta$  défini par

$$\Delta = \Psi^*(\dot{\mathbf{Z}}, \mathbf{Z}, T) + \phi^*(\dot{\mathbf{F}}, \mathbf{F}, T) - \chi(\mathbf{H}, T)$$

avec  $\Psi^*$ ,  $\phi^*$  et  $\chi$  les potentiels cinétiques, visqueux et de conduction et  $\mathbf{Z}$  représente les variables internes. On peut également définir  $\mathbf{Y}$  la force conjuguée à la déformation plastique cumulée:

$$\mathbf{Y} = -\frac{\partial W}{\partial \mathbf{Z}} = \frac{\partial \Psi^*}{\partial \dot{\mathbf{Z}}}$$

avec  $\Psi^*$  le pseudo-potential dual obtenue par la transformation de Legendre-Fenchel à partir de  $\Phi$  défini par:

$$\Psi^* = \sup_{\mathbf{Y}} \left\{ \mathbf{Y} \dot{\mathbf{Z}} - \Psi \right\} = \sup_{\mathbf{Y}} \left\{ \mathbf{Y} \frac{\partial \Psi}{\partial \mathbf{Y}} - \Psi \right\}$$

À partir de cela, on peut en déduire une formulation variationnelle proposée par Yang et al. [Yang et al., 2006]. Le potentiel d'un matériaux dissipatif standard est donné par:

$$\begin{aligned}\Phi \left( \dot{\phi}, T, \dot{\eta}, \dot{\mathbf{Z}} \right) &= \int_B \left[ \dot{E} - \rho_0 T \dot{\eta} + \Delta \left( \frac{T}{\Theta} \dot{\mathbf{F}}, \frac{T}{\Theta} \dot{\mathbf{Z}}, -\frac{1}{T} \nabla T \right) \right] dV \\ &\quad - \int_B \rho_0 \mathbf{B} \cdot \dot{\phi} dV - \int_{\partial_T B} \bar{\mathbf{T}} \cdot \dot{\phi} dS \\ &\quad + \int_B \rho_0 Q \log \frac{T}{T_0} dV - \int_{\partial_\eta B} \bar{H} \log \frac{T}{T_0} dS\end{aligned}$$

Deux températures,  $\Theta$  et  $T$  sont introduites, correspondant respectivement à une température *interne* ou d'*équilibre* et une température *externe* nécessaires pour retrouver les équations d'équilibre.

Au delà d'une approche variationnelle, on adopte également une approche incrémentale. De même que pour la Material Point Method (MPM), le système est décrit en utilisant des points matériels. Ces points sont suivis tout au long de la déformation. À un temps  $t$  donné, chaque point matériel a sa propre masse, masse volumique, vitesse, etc. nécessaire à la description du modèle constitutif. La discrétisation du potentiel se fait en deux temps: une discrétisation temporelle suivie d'une discrétisation spatiale.

En considérant un intervalle de temps  $[t_n, t_{n+1}]$ , on aboutit au potentiel incrémental suivant:

$$\begin{aligned}\Phi_n(\boldsymbol{\phi}_{n+1}, T_{n+1}) &= \inf_{\eta_{n+1}, \mathbf{Z}_{n+1}} \Phi_n(\boldsymbol{\phi}_{n+1}, T_{n+1}, \eta_{n+1}, \mathbf{Z}_{n+1}) \\ &= \int_B \psi_n(\mathbf{F}_{n+1}, T_{n+1}, \mathbf{G}_{n+1}) dV \\ &\quad - \int_B \rho_0 \mathbf{B}_{n+1} \cdot (\boldsymbol{\phi}_{n+1} - \boldsymbol{\phi}_n) dV - \int_{\partial_T B} \bar{\mathbf{T}}_{n+1} \cdot (\boldsymbol{\phi}_{n+1} - \boldsymbol{\phi}_n) dS \\ &\quad + \int_B \Delta t \rho_0 Q_{n+1} \log \frac{T_{n+1}}{T_n} dV - \int_{\partial_\eta B} \Delta t \bar{H}_{n+1} \log \frac{T_{n+1}}{T_n} dS\end{aligned}$$

avec

$$\begin{aligned}\psi_n(\mathbf{F}_{n+1}, T_{n+1}, \mathbf{G}_{n+1}) &= \inf_{\eta_{n+1}, \mathbf{Z}_{n+1}} [(E_{n+1} - E_n) - \rho_0 T_{n+1} (\eta_{n+1} - \eta_n) + \Delta t \Delta_{n+1}] \\ &= \mathcal{W}_n - \Delta t \chi(\mathbf{G}_{n+1})\end{aligned}$$

où  $\chi(\mathbf{G})$  est la fonction de dissipation de Biot telle que

$$\mathbf{H} = \frac{\partial \chi}{\partial \mathbf{G}}(\mathbf{G}) ; \quad \mathbf{G} \equiv -\frac{\nabla T}{T}$$

$\psi_n$  peut être vu comme une densité d'énergie thermo-élastique. Le problème variationnel se réduit alors à:

$$\inf_{\boldsymbol{\phi}_{n+1}, T_{n+1}} \sup \Phi_n(\boldsymbol{\phi}_{n+1}, T_{n+1})$$

La discrétisation spatiale fait intervenir les  $n_p$  points matériels ainsi que les fonctions d'interpolation  $N_a$  associées aux  $n_{nodes}$  nœuds  $\mathbf{x}_a$ .

$$\boldsymbol{\phi}^h(\mathbf{x}) = \sum_{a=1}^{n_{nodes}} N_a(\mathbf{x}) \mathbf{x}_a$$

$$T^h(\mathbf{x}) = \sum_{a=1}^{n_{nodes}} N_a(\mathbf{x})T_a$$

où  $\mathbf{x}_a$  et  $T_a$  sont les températures aux nœuds. Le principe variationnel réduit devient alors:

$$\inf_{\phi_{n+1}^h, T_{n+1}^h} \sup \Phi_n(\phi_{n+1}^h, T_{n+1}^h; \phi_n, T_n, \mathbf{Z}_n)$$

Par substitution dans l'expression du potentiel incrémental et en enlevant les indices  $n + 1$  pour plus de lisibilité, on obtient:

$\forall \delta \mathbf{x}_a$  admissible,

$$\sum_{a=1}^{n_{nodes}} \mathbf{f}_a \cdot \delta \mathbf{x}_a = \sum_{a=1}^{n_{nodes}} \left\{ \int_B [\mathbf{P}^h \cdot \nabla N_a - \rho_0 \mathbf{b} N_a] dV - \int_{\partial_T B} \bar{T} N_a dS \right\} \cdot \delta \mathbf{x}_a = 0$$

$\forall \delta T_a$  admissible,

$$\sum_{a=1}^{n_{nodes}} Q_a \delta T_a = \sum_{a=1}^{n_{nodes}} \left\{ \int_B \left[ -\rho_0 \Delta \eta_{eff}^h N_a + \Delta t \mathbf{H}^h \cdot \left( \frac{\nabla N_a}{T^h} + \frac{\mathbf{G}^h}{T^h} N_a \right) + \Delta t \rho_0 Q \frac{N_a}{T_n} \right] dV - \int_{\partial \eta} \Delta t \bar{H} \frac{N_a}{T_n} dS \right\} \cdot \delta T_a = 0$$

où  $\mathbf{P}^h$ ,  $\Delta \eta_{eff}^h$  et  $\mathbf{H}^h$  sont respectivement définis par:

$$\begin{aligned} \mathbf{P}^h &= \frac{\partial \mathcal{W}_n}{\partial \mathbf{F}}(\mathbf{F}^h, T^h; \mathbf{F}_n, T_n, \mathbf{Z}_n) \\ \rho_0 \Delta \eta_{eff}^h &= -\frac{\partial \mathcal{W}_n}{\partial T}(\mathbf{F}^h, T^h; \mathbf{F}_n, T_n, \mathbf{Z}_n) \\ \mathbf{H}^h &= \frac{\partial \chi}{\partial \mathbf{G}}(\mathbf{G}^h; \mathbf{F}_n, T_n, \mathbf{Z}_n) \end{aligned}$$

On obtient ainsi les équilibres mécaniques et thermiques:

$$\begin{cases} \mathbf{f}_a^{\text{int}} - \mathbf{f}_a^{\text{ext}} = 0 \\ Q_a^{\text{int}} - Q_a^{\text{ext}} = 0 \end{cases} \quad (a = 1, \dots, n_{nodes})$$

En considérant les fonctions de forme Local Maximum Entropy définies sur le domaine  $B$  (ou une restriction du domaine  $B$ ), les forces et flux internes et externes

sont donnés par:

$$\begin{aligned}\mathbf{f}_a^{\text{int}} &= \int_B \mathbf{P}^h \cdot \nabla N_a \, dV \\ \mathbf{f}_a^{\text{ext}} &= \int_B \rho_0 \mathbf{b} N_a \, dV + \int_{\partial_T B} \bar{T} N_a \, dS \\ Q_a^{\text{int}} &= \int_B \left[ \rho_0 \Delta \eta_{eff}^h N_a - \Delta t \mathbf{H}^h \cdot \left( \frac{\nabla N_a}{T^h} + \frac{\mathbf{G}^h}{T^h} N_a \right) \right] dV \\ Q_a^{\text{ext}} &= \int_B \Delta t \frac{\rho_0 Q}{T_n} N_a \, dV - \int_{\partial_{\text{eta}} B} \Delta t \frac{\bar{H}}{T_n} N_a \, dS\end{aligned}$$

### 3 Cadre variationnel et modèles d'écoulements

Le modèle de Johnson-Cook [Johnson and Cook, 1983] est l'un des modèles les plus utilisés pour décrire l'écoulement d'un solide comme une fonction de la vitesse de déformation plastique  $\dot{p}$ , de la température  $T$  et de la déformation plastique équivalente  $p$ . Ce principe est défini empiriquement par:

$$\sigma_y(p, \dot{p}, T) = \left[ A + B p^{\frac{1}{n}} \right] \left[ 1 + C \ln \left( \frac{\dot{p}}{\dot{p}_0} \right) \right] \left[ 1 - (T^*)^{\frac{1}{m}} \right]$$

où  $A, B, C, n$  et  $m$  sont des constantes du matériaux. La température normalisée  $T^*$  est définie par

$$T^* = \begin{cases} 0 & \text{if } T < T_0 \\ \frac{T - T_0}{T_m - T_0} & \text{if } T_0 < T < T_m \\ 1 & \text{if } T > T_m \end{cases}$$

Dans le cadre variationnel, les énergies stockées et dissipées en chaleurs sont exprimées par:

$$W^p(p, T) = \left( A_s p + \frac{B_s}{n+1} p^{n+1} \right) \left( 1 - (T^*)^{\frac{1}{m}} \right)$$

$$\Psi^*(\dot{p}; p, T) = \left[ (A_d + B_d p^n) \dot{p} + (A + B p^n) C \dot{p}_0 \left( \frac{\dot{p}}{\dot{p}_0} \ln \left( \frac{\dot{p}}{\dot{p}_0} \right) - \frac{\dot{p}}{\dot{p}_0} + 1 \right) \right] \left( 1 - (T^*)^{\frac{1}{m}} \right)$$

où  $A_s + A_d = A$  et  $B_s + B_d = B$ .

# Résumé du chapitre 3

## Interpolation Local Maximum Entropy

### 1 Théorie de l'information

La théorie de l'information [?] est une théorie probabiliste utilisée pour quantifier l'information contenue dans une donnée. Une des caractéristiques de cette théorie est qu'elle donne un sens physique à la notion d'information: laissée à elle-même, l'information ne peut qu'évoluer vers la désorganisation, c'est-à-dire vers un accroissement d'entropie. Cette entropie est définie par Shannon par:

$$H(p) = -p \log(p)$$

où  $p$  est la probabilité associée à un événement donné. Dans le cas où on considère un ensemble de  $n$  événements  $i$  associés aux probabilités  $p_i$ , l'entropie s'écrit:

$$H(p_1, \dots, p_n) = - \sum_{i=1}^n p_i \log(p_i)$$

Lorsqu'on souhaite représenter une connaissance imparfaite par une loi de probabilité, le principe d'entropie maximale consiste à identifier des contraintes auxquelles cette distribution doit répondre et, parmi toutes les distributions possibles, choisir celle qui a la plus grande entropie. De toutes ces distributions, c'est celle d'entropie maximale qui contient le moins d'informations, et elle est donc pour cette raison la moins arbitraire de toutes celles que l'on pourrait utiliser.

### 2 Problème Local Maximum Entropy

Ce chapitre est basé sur l'article [Arroyo and Ortiz, 2006]. La construction de l'interpolation Local Maximum Entropy peut être retrouvée intégralement dans cet

article.

Considérons un ensemble de nœuds  $X$  et le plus petit convexe  $\text{conv}X$  contenant  $X$ . Un lien a été établi entre l'entropie de Shannon et la construction de fonctions d'interpolation pour un ensemble de nœuds donné. Tout d'abord, on souhaite utiliser une distribution de probabilité la moins arbitraire possible. Il convient donc de considérer la solution  $p = [p_1, \dots, p_n]$  du problème Maximum Entropy:

$$\text{(ME) Maximiser} \quad H(\mathbf{p}) = - \sum_{a=1}^n p_a \log p_a$$

$$\begin{aligned} \text{tel que } \forall \mathbf{x} \in \text{conv}X \quad & p_a(\mathbf{x}) \geq 0, a = 1, \dots, N \\ & \sum p_a(\mathbf{x}) = 1 \\ & \sum p_a(\mathbf{x}) \mathbf{x}_a = \mathbf{x} \end{aligned}$$

Ce problème a une solution si, et seulement si,  $\mathbf{x} \in \text{conv}X$ . De plus la solution est alors unique. Ces fonctions, bien qu'optimales au sens de la théorie de l'information, sont non-locales et peu efficaces en tant que fonctions d'interpolation pour de la mécanique numérique. Pour améliorer leur efficacité, il est nécessaire d'ajouter une notion de localité: ce qui se passe près du point  $\mathbf{x}$  considéré est le plus intéressant. Cette notion de localité peut être directement reliée à la largeur des fonctions  $p_a$ :

$$\text{(RAJ) Minimiser} \quad U(\mathbf{x}, \mathbf{p}) \equiv \sum_{a=1}^n p_a |\mathbf{x} - \mathbf{x}_a|^2$$

$$\begin{aligned} \text{tel que } \forall \mathbf{x} \in \text{conv}X \quad & p_a(\mathbf{x}) \geq 0, a = 1, \dots, N \\ & \sum p_a(\mathbf{x}) = 1 \\ & \sum p_a(\mathbf{x}) \mathbf{x}_a = \mathbf{x} \end{aligned}$$

Une solution à ce problème existe et est unique sauf cas dégénéré. En général, il n'est pas possible de trouver une solution commune aux deux problèmes. On cherche donc une solution Pareto optimale, c'est-à-dire qui représente le meilleur compromis entre les deux solutions, ce qui revient donc à résoudre le problème Local Maximum Entropy:

$$\text{(LME)}_\beta \text{ Minimiser} \quad f_\beta(\mathbf{x}, \mathbf{p}) \equiv \beta U(\mathbf{x}, \mathbf{p}) - H(\mathbf{p})$$

$$\begin{aligned} \text{tel que } \forall \mathbf{x} \in \text{conv}X \quad & p_a(\mathbf{x}) \geq 0, a = 1, \dots, N \\ & \sum p_a(\mathbf{x}) = 1 \\ & \sum p_a(\mathbf{x}) \mathbf{x}_a = \mathbf{x} \end{aligned}$$

Pour  $\beta \in ]0; +\infty[$ , la solution du problème  $(\text{LME})_\beta$  est Pareto optimale. Pour  $\beta = 0$ , la solution est unique et est la solution du problème (ME). Pour  $\beta = +\infty$ , les solutions sont les solutions du problème (RAJ).

On montre que la solution au problème  $(LME)_\beta$  est donnée par:

$$p_{\beta a}(\mathbf{x}) = \frac{1}{Z(\mathbf{x}, \boldsymbol{\lambda}^*(\mathbf{x}))} \exp [-\beta |\mathbf{x} - \mathbf{x}_a|^2 + \boldsymbol{\lambda}^* \cdot (\mathbf{x} - \mathbf{x}_a)]$$

où:

$$\boldsymbol{\lambda}^*(\mathbf{x}) = \arg \min_{\boldsymbol{\lambda} \in \mathbb{R}^d} \log Z(\mathbf{x}, \boldsymbol{\lambda})$$

$$Z(\mathbf{x}, \boldsymbol{\lambda}) = \sum_{a=1, \dots, n} \exp [-\beta |\mathbf{x} - \mathbf{x}_a|^2 + \boldsymbol{\lambda} \cdot (\mathbf{x} - \mathbf{x}_a)]$$

De plus, le minimiseur  $\boldsymbol{\lambda}^*(\mathbf{x})$  est unique.

En pratique, on utilisera le paramètre adimensionnel  $\gamma = h^2\beta$  où  $h$  est une longueur caractéristique locale de  $X$ .

### 3 Quelques caractéristiques

Ces fonctions d'interpolation présentent plusieurs avantages. Tout d'abord, elles permettent l'application de conditions de Dirichlet sur les bords de  $convX$ , ce qui est nécessaire pour la modélisation du procédé RFW. Les fonctions d'interpolation étant des exponentielles, elles ne sont jamais rigoureusement nulles au bord. Cependant, leur évaluation est suffisamment petite (entre  $10^{-7}$  et la précision machine) pour être considérée nulle.

Une autre caractéristique est la positivité du jacobien, qui est l'hessien du  $\log Z$ , dans le calcul de  $\boldsymbol{\lambda}^*$ .

### 4 Points matériels

En théorie, les points matériels peuvent être placés n'importe où. Cependant, certaines positions sont peut-être plus intéressantes que d'autre d'un point de vue numérique. Une première idée a été de chercher les positions  $\mathbf{x}$  telles que  $\boldsymbol{\lambda}^*(\mathbf{x}) = 0$ . En effet, les fonctions d'interpolations ne sont évaluées qu'aux points matériels et  $\boldsymbol{\lambda}^*(\mathbf{x}) = 0$  signifie que les conditions du problème  $(LME)_\beta$  sont naturellement vérifiées. De plus, si on sait que  $\boldsymbol{\lambda}^*(\mathbf{x}) = 0$ , il n'y a alors plus besoin de le calculer, ce qui représente un gain en temps de calcul.

Cependant, il apparaît que trouver les positions adéquates n'est pas simple parce qu'elles dépendent du nuage de nœuds. Il est possible de les trouver mais cela coûte aussi cher, si ce n'est plus, en temps de calcul que le calcul de  $\boldsymbol{\lambda}$ . Cette démarche a donc été abandonnée.



Il a été arbitrairement décidé que les points matériels devaient être bien répartis sur le nuage de nœuds. Une triangulation de Delaunay a été effectuée sur le nuage de nœuds pour obtenir une liste de simplex sur lesquels les points matériels seront disposés. Ainsi, les quadratures usuelles ont été testées et comparées: Gauss-Legendre (utilisé pour placer les points de Gauss dans la méthode des éléments finis), Gauss-Hermite (recommandée pour l'intégration de fonctions du type  $\exp(-x^2)$ ) et une quadrature par subdivision. Il apparaît que la quadrature de Gauss-Legendre donne le meilleur compromis en terme de convergence et de précision pour un faible nombre de points matériels.

# Résumé du chapitre 4

## Validation

### 1 Patch test

Le classique patch test est utilisé pour contrôler la précision de notre implémentation. Ce patch test est présenté par la figure 4.1 à la page 69.

La transformation suivante est imposée aux nœuds sur le contour:

$$\begin{pmatrix} 1 & -\sqrt{3}/2 \\ \sqrt{3} & 1/2 \end{pmatrix}$$

En fonction de  $\gamma$  et de la configuration des nœuds, on compare l'erreur faite sur le gradient de déformation aux points matériels:

	Regular node-set (a)		Irregular node-set (b)			
mPt	$\gamma = 9.44$	$\gamma = 1.8$	$\gamma = 9.44$	$\gamma = 8.02$	$\gamma = 1.8$	$\gamma = 0.8$
1	6.50e-10	1.29e-5	7.51e-2	7.22e-2	4.16e-2	5.30e-2
3	4.18e-11	2.86e-4	2.37e-3	1.54e-3	3.01e-2	5.88e-2

**Table:** Erreur relative sur le gradient de déformation aux points matériels.

Il apparaît que la précision dépend énormément de la position des nœuds mais aussi de  $\gamma$ . Dans le cas d'un nuage de nœuds régulier, il est possible d'obtenir une très bonne précision sur le résultat.

### 2 Barre de Taylor

Notre implémentation est maintenant testé sur le très classique cas test de la barre de Taylor dans un contexte axy-symétrique. Les conditions limites de ce cas test sont présenté par la figure 4.3 à la page 70.

Une vitesse initiale est appliquée à chaque nœud égale à  $227 \text{ m.s}^{-1}$ .

L'apparition ou non de verrouillage numérique (dit locking), est étudiée en fonction de la densité des nœuds dans la barre, la densité de points matériels ainsi que de  $\gamma$ . Il apparaît qu'il est possible d'éviter le phénomène de locking en utilisant un  $\gamma$  petit et une densité de points matériels suffisamment faible.

La table ci-dessous compare nos résultats à d'autres résultats issus de la littérature:

	Final length (mm)	Final mushroom radius (mm)	Max. effective plastic strain	Max von Mises stress (MPa)
[Kamoulakos, 1990]	21.47 – 21.66	7.02 – 7.12	2.47 – 3.24	472 – 476
[Zhu and Cescotto, 1995]	21.26 – 21.49	6.89 – 7.18	2.75 – 3.03	419 – 477
[Camacho and Ortiz, 1997]	21.42 – 21.44	7.21 – 7.24	2.97 – 3.25	/
OTM [Li, 2009]	21.43	6.8	3.0	474
LME	21.50	6.81	2.69	516

**Table:** Cas test de la barre de Taylor: comparaison des résultats.

Notre implémentation permet de retrouver les résultats de la littérature et d'éviter le locking.

### 3 Contact unilatéral

Une modélisation du contact unilatéral entre un solide déformable et un corps rigide a été implémenté en utilisant une méthode par pénalité. Cette implémentation a été testée en réutilisant le cas de la barre de Taylor: axy-symétrie et même vitesse initiale. Les nœuds initialement bloqués sur la direction de la vitesse initiale sont maintenant soumis à une condition de contact.

La méthode de pénalité consiste à évaluer une fonction de pénétration (gap) et à appliquer un effort dépendant de ce gap aux nœuds qui ont pénétré le corps rigide. L'évolution du gap et de l'effort de pénalité en fonction du choix de coefficient de pénalité sont présentés par les figures 4.12 à la page 77 et 4.14 à la page 78.

Le rebond dû à la pénalité est de l'ordre de  $0.25 \text{ mm}$ , ce qui est considéré comme acceptable. De même, la pénétration résiduelle présente après le rebond est suffisamment faible pour être considérée comme acceptable.

## 4 Conduction thermique

Afin de tester la capacité de notre implémentation à gérer la conduction thermique, un problème de thermique pure est abordée et est présenté dans la figure 4.15 à la page 79.

Les nœuds d'un côté de la plaque considérée sont à une température imposée  $T_0 = 500K$  alors que les autres sont initialement à  $T = 0$ . La figure 4.20 à la page 81 compare l'évolution de la température aux nœuds indiqués avec les résultats obtenus avec la méthode des éléments finis en utilisant le logiciel Abaqus (v.6.10).

Les résultats obtenus sont identiques: notre implémentation passe donc ce test.

## 5 Convection

Enfin, notre implémentation est testée sur un cas de couplage thermo-mécanique en grande déformation impliquant un phénomène de convection. Les conditions limites et le chargement sont donnés sur la figure 4.21 à la page 83.

Le cylindre et l'air extérieur sont tous deux à  $T_0 = 293K$ . Une déformation totale de 50% est imposée au cylindre dans le sens de sa hauteur. À cause des déformations plastiques, la température a tendance à augmenter de manière homogène dans la pièce. Cependant, les conditions de convection apporte une hétérogénéité qui fait que les nœuds au centre du cylindre seront les plus chauds, ce qui entraîne des déformations plus importantes au centre. En utilisant un nuage de nœuds suffisamment dense, on obtient le résultat donné par la figure 4.23 à la page 84.

Les résultats sont conformes à ce qui est attendu et correspondent aux résultats obtenus avec Abaqus en utilisant la MEF. Notre implémentation réussit donc ce test.



# Résumé du chapitre 5

## Modélisation du procédé de soudage par friction rotative

### 1 Hypothèses

Quelques hypothèses sont faites pour modéliser le procédé de soudage par friction rotative (RFW).

La simulation est effectuée en 2D axy-symétrique. En effet, on suppose que les différents champs et variables internes ne dépendent pas de la position angulaire, ce qui est vérifié dans la réalité. Le calcul est effectué en quasi-statique, supposant ainsi que les effets d'inertie sont négligeables.

On suppose que le contact entre les deux pièces à souder est un plan défini par  $y = 0$ . On modélise alors le contact entre un solide déformable et un corps rigide. Cette hypothèse implique que le matériau soit homogène et qu'il n'y ait pas de pénétration locale d'une des pièces soudées dans l'autre, ce qui est automatique vérifié si les deux pièces sont faites du même matériaux. Si les deux matériaux sont différents mais qu'ils sont tout deux homogènes, cette hypothèse est encore valable.

La loi de friction utilisée est la classique loi de Coulomb avec un coefficient de friction dépendant de la température. Les efforts tangents à l'interface de soudage sont négligés au vu des autres efforts modélisés.

On considère que la puissance de friction est intégralement transformée en chaleur. On suppose que cette chaleur se répartit équitablement dans les deux pièces si le matériau utilisé est le même (la répartition dépend de l'effusivité des deux matériaux).

Enfin, la convection entre l'air et la pièce modélisée n'est pas prise en compte. La configuration initiale est schématisée par la figure 5.1 à la page 90.

## 2 Surface équivalente représentée par un nœud

Afin de pouvoir prendre en compte la densité de nœud à l'interface de soudage pour le calcul des efforts de pénalité et de l'apport de chaleur en découplant, il est nécessaire d'avoir une surface équivalente représentée par chaque nœud. En utilisant la méthode des éléments finis, cette question ne se pose pas dans la mesure où la notion d'élément même permet d'avoir cette surface mais dans le cas d'une méthode sans maillage, il n'y a pas d'équivalent. Pour ce faire, on définit la surface équivalente comme montrée par la figure 5.3 à la page 91.

Dans la suite, on suppose que le nuage de nœuds est régulier. On définit alors pour chaque nœud la longueur caractéristique  $h_a$  représentant la distance locale caractéristique entre deux nœuds:

$$h_a = \min \{ \|\mathbf{x}_a - \mathbf{x}_n\|_2 \mid n \in \llbracket 1; N \rrbracket \} \quad (\text{A.10})$$

où  $N$  est le nombre total de nœud. Les longueurs représentées en rouge précédemment sont alors égales au  $h_a$  du nœud considéré. En prenant en compte que nous travaillons en axy-symétrie, on peut alors en déduire une surface d'échange avec l'extérieur. C'est cette surface qui est utilisée dans le calcul des effort des pénalité (le coefficient de pénalité est alors une raideur surfacique).

## 3 Friction, chaleur et formulation symétrique du frottement

On suppose que la puissance de friction est intégralement transformée en chaleur. L'apport de chaleur (en  $J.K^{-1}$ ) au nœud considéré est alors exprimé par:

$$\Phi = -\frac{1}{2} \frac{1}{T_0} \cdot \nu \cdot k_{S_a} S_a g_a \cdot \omega r_a \cdot dt$$

avec le facteur  $\frac{1}{2}$  traduisant la répartition de chaleur entre les deux pièces (dans le cas de deux matériaux identiques,  $\frac{b_1}{b_1+b_2} = \frac{1}{2}$  avec  $b_1$  et  $b_2$  les effusivités respectives des matériaux),  $T_0$  la température initiale,  $\nu$  le coefficient de frottement dépendant de la température,  $k_{S_a} S_a g_a$  l'effort de pénalité où  $g_a$  est le gap calculé,  $\omega$  la vitesse de rotation,  $r_a = x_a$  l'abscisse du nœud  $a$  et  $dt$  l'incrément de temps actuel.

Cependant, ce chargement thermique ne peut pas être utilisé tel quel dans un environnement variationnel. En effet, certains termes doivent être ajoutés à la matrice

tangente. Celle-ci est symétrique ; or il apparaît que :

$$\begin{aligned} \frac{\partial \Phi}{\partial x} \neq 0 \text{ alors que } \frac{\partial F^t}{\partial T} = 0 \\ \frac{\partial \Phi}{\partial y} \neq 0 \text{ alors que } \frac{\partial F^n}{\partial T} = 0 \end{aligned}$$

Pour résoudre ce problème, nous avons choisi de *retarder* le flux de chaleur. En effet, au lieu d'utiliser la position  $x_a^n$  du pas de temps  $n$  actuel, nous utilisons la position  $x_a^{n-1}$  et le gap  $g_a^{n-1}$  du pas de temps précédent. Si le pas de temps est suffisamment petit, ce qui est le cas dans ce genre de simulation, l'erreur commise est négligeable. Grace à cette astuce,  $\Phi$  est maintenant indépendant de la position  $x_a^n$  et on a bien:

$$\frac{\partial^{n+1} \Phi}{\partial x^{n+1}} = \frac{\partial^{n+1} \Phi}{\partial y^{n+1}} = 0$$

## 4 Modélisation du procédé

La pièce à souder ici est un cylindre creux. La partie modélisée de cette pièce est représentée par la figure 5.4 à la page 98.

Puisqu'on considère le plan de contact comme étant rigide et fixe, il convient d'appliquer un charment égal à 50% de celui utilisé lors de la campagne d'essai. On admet aussi que les degrés de liberté correspondant au déplacement vertical des nœuds en rouge sont liés.

Tout d'abord, il faut noter qu'actuellement, le code arrête de converger après 10.6 s. De ce fait, il n'est possible d'analyser le résultat que sur cet intervalle de temps.

Au début de la simulation, le champs de température prends une forme de cône dont l'axe est contenu dans le plan de contact et qui s'élargit quand on s'éloigne de l'axe d'axy-symétrie, ce qui correspond bien au résultat voulu. Au bout d'environ 3 s, les nœuds aux extrémités commencent à s'éloigner du plan de contact, commençant ainsi la formation du flash. À ce moment là, le nœud où la température est la plus élevée est au centre de la pièce, au niveau du plan de contact. À  $t \simeq 8.5$  s, la température de ce nœud dépasse la température de fusion du matériau (ici de l'acier P265GH), ce qui ne se produit pas dans la réalité: la température est sensée être proche de la température de fusion mais pas la dépasser. Ceci peut être une des causes de non convergence du code.

Les autres causes peuvent être la loi d'écoulement non adaptée, la modélisation du contact trop simpliste, la non prise en compte de la convection qui aurait tendance à refroidir la pièce, évitant peut être ainsi la température supérieure à celle de fusion, la non prise en compte des changements de phase ou encore une implémentation



pas assez robuste pour ce genre de simulation numériquement très exigeante.

Si on compare nos résultats aux données expérimentales, on se rend compte que les résultats obtenus sont prometteurs. En effet, la zone affectée thermiquement (ZAT) est conforme à ce qu'elle est en réalité. De même, la formation du flash correspond à l'évolution constatée dans la littérature pour la même catégorie de soudage.

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# Thèse de Doctorat

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On a Local Maximum Entropy interpolation approach for simulation of coupled thermo-mechanical problems. Application to the Rotary Frictional Welding process.

À propos d'une approche Local Maximum Entropy pour la simulation de problèmes thermo-mécaniques. Application au procédé de soudage par friction rotative.

This work aims at developing a meshless method based on Local Maximum Entropy interpolation and material points integration for the simulation of coupled thermo-mechanical problems. This work has been motivated by the industrial need to develop an innovative thermo-mechanical modeling of the Rotary Frictional Welding process. The main advantage of using a meshless method is that it is possible to deal with large deformation without loss of accuracy. Using a LME interpolation approach allows to put Dirichlet boundary conditions at the boundary, which is generally very difficult to do with a non-grid based method. A special attention was paid to the position of the material points in order to obtain the best compromise in term of convergence rate, calculation time and accuracy of the result. An incremental variational framework is used to derive the state and the evolution equations of coupled boundary-values problems. The incremental aspect leads to a temporal discretization of the governing equation to reduce time-dependent problems to a sequence of incremental problems each characterized by an optimality (saddle-point) principle. Then, a spatial discretization is applied to describe locally the state of the material using the material points. The evolution of the system can be described using either an explicit or an implicit scheme. The proposed meshless method is tested through a series of benchmark tests which involve coupled thermo-mechanical phenomena considering large deformations and contact.

**Keywords:** Meshless method, Local Maximum Entropy basis function, thermo-mechanics, incremental variational approach.

Ce travail vise à développer une méthode *meshless* basée sur une interpolation de type Local Maximum Entropy avec intégration sur des points matériels afin de simuler des problèmes thermo-mécaniques. La motivation de ce travail a été le besoin industriel d'avoir une modélisation thermo-mécanique innovante du procédé de soudage par friction rotatif. L'intérêt d'utiliser une méthode *meshless* est de pouvoir traiter des problèmes en grandes déformations sans perte de précision. En outre, l'interpolation LME permet d'appliquer des conditions de Dirichlet aux bords, ce qui est en général difficile avec les méthodes *meshless*. Une attention particulière a été portée sur la position des points matériels afin d'obtenir un compromis entre vitesse de convergence, temps de calcul et précision du résultat. Les problèmes considérés seront appréhendés avec une vision lagrangienne à l'aide d'une approche énergétique variationnelle incrémentale menant à une discrétisation temporelles afin de réduire les problèmes dépendant du temps à une succession de problèmes incrémentaux, chacun caractérisé par un principe d'énergie optimal (point de selle). S'ensuit une discrétisation spatiale pour décrire localement l'état du matériau via l'utilisation de points matériels. L'évolution du système peut être traitée de manière explicite ou implicite. La méthode proposée est testée sur une série de cas test mettant en jeu des phénomènes de couplage thermo-mécanique en considérant des grandes déformations et du contact.

**Mots clés :** Méthode sans maillage, Interpolation de type Local Maximum Entropy, Thermo-mécanique, Approche énergétique variationnelle incrémentale.